

CALCULATION SHEET

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Rectangular Tank on Ground

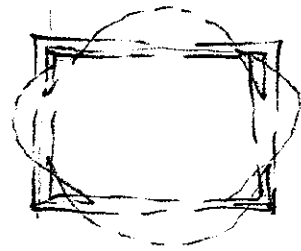
Subjected to bending in 2 directions

- 1) Horizontal Strip
- 2) Vertical Cantilevers.

$L/S < 2.00$ \Rightarrow Design as continuous horizontal slab & subjected to water pressure of wh /unit area at depth h from the water surface

$$M_{@ \text{centre}} = \frac{wh \cdot L^2}{16}$$

$$M_{@ \text{corner}} = \frac{wh \cdot L^2}{12}$$



$L/S \geq 2.00$ \Rightarrow a) long wall are consider as vertical cantilevers.

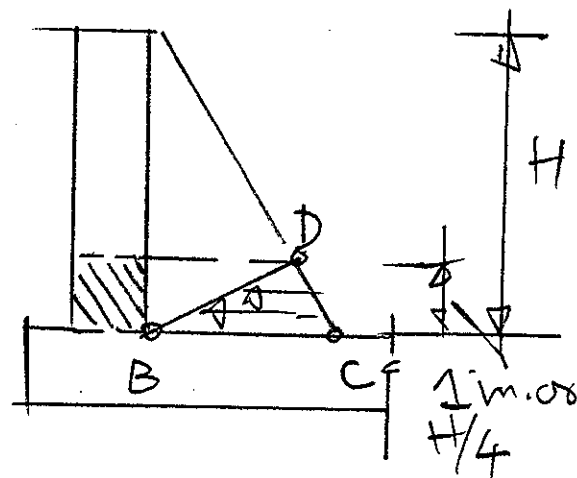
Short wall consider as spanning between the long wall and taken as fixed

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b) Cantilever effect of Short wall

- Though the short wall in principle spans horizontally between long wall
- It possible that short wall may have a cantilevering effect.
- Usual to consider that for the bottom 1 meter or $H/4$
- The water pressure taken for design on cantilevering part is given by BDC

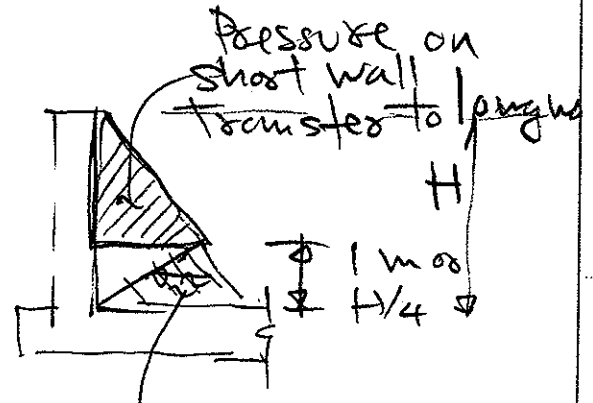
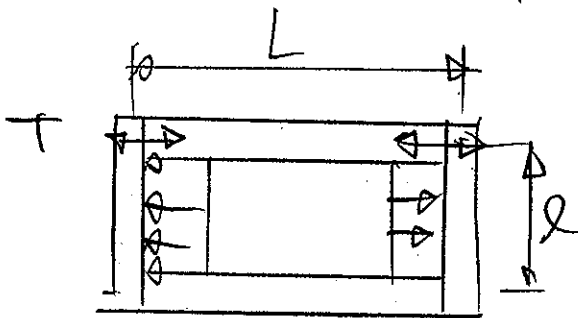


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c) Pull on the long wall - Short walls

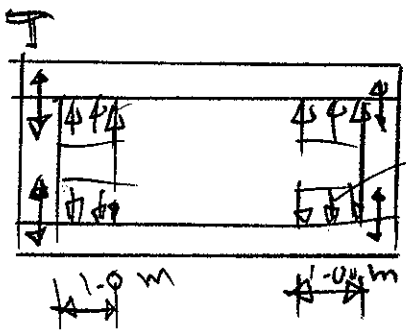
C.1) Pull on the long wall



Considers that the bottom will act as a cantilever.

$$M = \frac{W(H-1)L}{2}$$

C.2) Pull on the short wall



long wall will behave actually as cantilever from Base Slab
Part of the water pressure act on the long wall for a certain distance from corners will produce a tension in the short wall

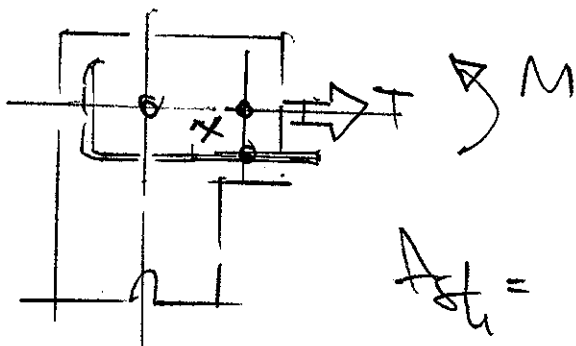
$$M = W(H \times 1) \times 1$$

per metre

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Analysis of Tank wall section
Subjected to bending & Pull



$$A_{st1} = \frac{M - T_x}{f_s j d}$$

$$A_{st2} = \frac{I}{f_s}$$

* Total requirement = $A_{st1} + A_{st2}$

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Water tank 5.00 x 5.00 x 3.00 m deep.

Thickness of wall

- i .15 cm
- ii 3 cm/metre depth + 5 = 9 + 5 = 14 cm
- iii 6 cm/metre length = 6 x 5 = 30 cm.

Provide Wall Thickness 30 cm.

Effective span = 5 + 0.30 = 5.30 m

design as continuous horizontal slab

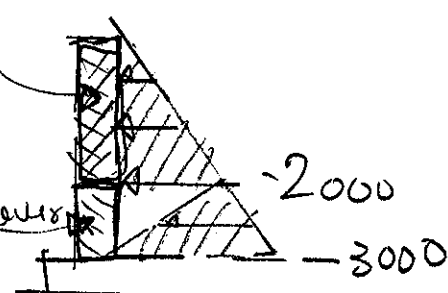
Consider a level 1 m above the floor

$P = 1000 \times 2 = 2000 \text{ kg/m}^2$

$M_{\text{corner}} = \frac{2000 \times 5.3^2}{12} = 4681 \text{ kg.m}$

$M_{\text{mid}} = \frac{2000 \times 5.3^2}{16} = 3511 \text{ kg.m}$

design as cantilever



Load on the wall per metre height at this level

$P = W(H-1) \frac{1}{2} = 1000(3-1) \frac{1}{2} = 5000 \text{ kg.}$

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Design of Corner Section

$$d = 30 - 3.4 = 26.6 \text{ cm}$$

$$\begin{aligned} \text{Effective } BM &= M - T \cdot x \\ &= 4681 - 5000 (0.266 - 0.15) \\ &= 4010 \text{ kg-m} \end{aligned}$$

$$A_{st1} = \frac{4010}{f_s \cdot j \cdot d} = 18.35 \text{ cm}^2$$

$$A_{st2} = \frac{5000}{f_s} = 5.00 \text{ cm}^2$$

$$A_{st} = 18.35 + 5.00 = 23.35 \text{ cm}^2 = \underline{\underline{\phi 19 @ 100 \text{ (A)}}}$$

(Horizontal)

Design of Mid-span Section

$$\begin{aligned} \text{Effective } BM &= 3511 - 5000 (0.266 - 0.15) \\ &= 2931 \text{ kg-m} \end{aligned}$$

$$A_{st1} = \frac{2931}{f_s \cdot j \cdot d} = 10.25 \text{ cm}^2$$

$$A_{st2} = \frac{5000}{f_s} = 4.00 \text{ cm}^2$$

$$A_{st} = 10.25 + 4.00 = 14.25 \text{ cm}^2 = \underline{\underline{\phi 19 @ 170 \text{ (B)}}}$$

(Horizontal)

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Design of the bottom 1 metre height of wall

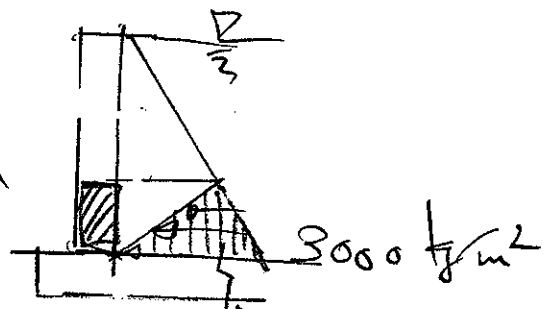
$$P = \frac{1}{2} \times 3000 = 1500 \text{ kg/m}$$

$$M = \frac{1}{3} \times 1500 = 500 \text{ kg-m/m}$$

$$A_{st} = \frac{500}{f_{s,d}} = 2.24 \text{ cm}^2/\text{m}$$

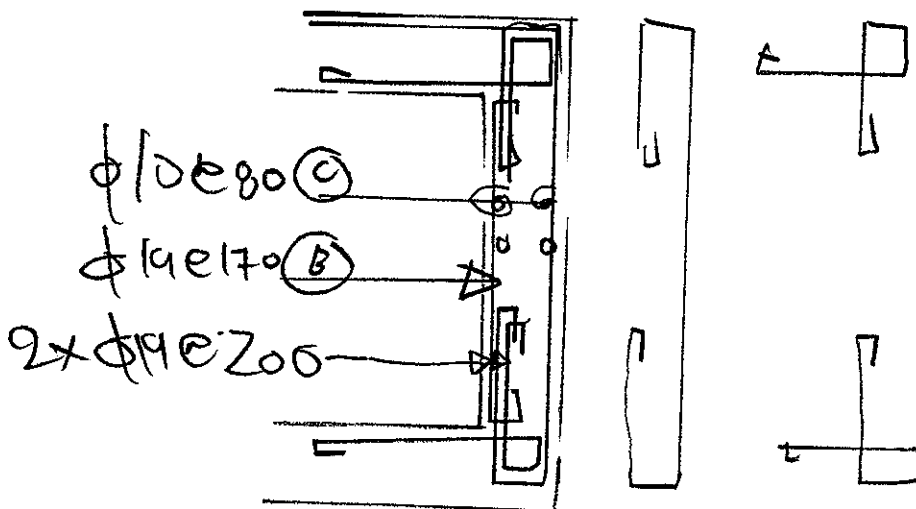
$$A_{min} = \phi 25 \times 100 \times 30 = 7.50 \text{ cm}^2/\text{m}$$

} $\phi 10 @ 80$ (C)
(Vertical)



Base slab

Provided 24 cm thick = $\phi 10 @ 80$ #
(Top)



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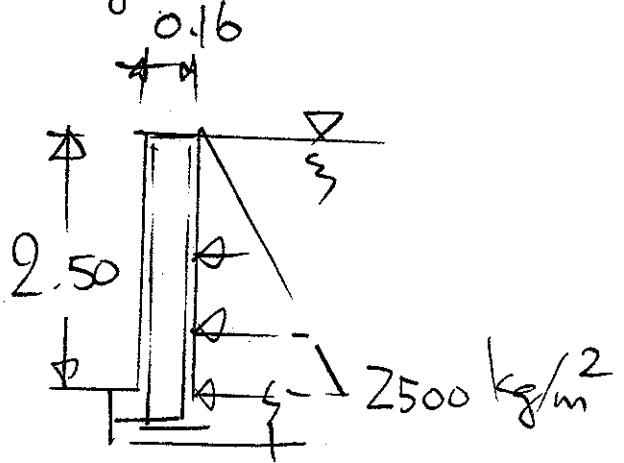
Water Tank 6.00 x 3.00 x 2.50 m deep.

$4l = \frac{6}{3} = 2.00$ - long wall designed as vertical cantilever
 - Short wall as spanning horizontal between long wall

Design of long wall

$$M = \frac{1}{2} \times 2500 \times 2.5 \times \frac{2.5}{3} = 2605 \text{ kg-m/m}$$

$$A_s = \frac{2605}{f_s j d} = 20.28 \text{ cm}^2/\text{m} = \phi 16 @ 90 \text{ mm}$$



Pull on the long wall (Vert.)

$$P = W(2.50 - 1) \times 3 = 2250 \text{ kg}$$

$$A_s = \frac{2250}{f_s} = 2.02 \text{ cm}^2/\text{m}$$

$$A_{st} = 0.25\% = 4.00 \text{ cm}^2/\text{m}$$

$$\phi 10 @ 160 \text{ mm} \text{ (Horz)}$$

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Design of Shost Wall

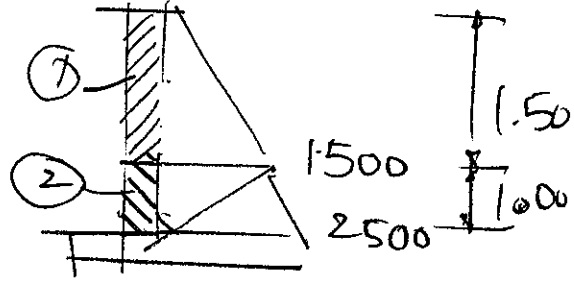
Considers a level one metre above the top of Bottom -

$$= 1000(2.50-1)$$

$$= 1500 \text{ kg/m}^2$$

Effective span of horizontal spanning slab.

$$= 3.00 + 0.16 = 3.16 \text{ m}$$



Corner Section

$$M_{\text{cor}} = 1500 \times 3.16^2 / 12$$

$$= 1249 \text{ kgam/m}$$

Water pressure at the level on 1-metre length of long wall will be transferred to the shost wall

$$= 1500 \times 1.00 = 1500 \text{ kg}$$

$$M = M_{\text{cor}} - T \cdot x$$

$$= 1249 - 1500(0.27 - 0.08)$$

$$= 1060 \text{ kgam}$$

$$A_{st} = \frac{1060}{f_s j d} = 9.94 \text{ cm}^2/\text{m}$$

$$A_{st} = \frac{1500}{f_s} = 1.35 \text{ cm}^2/\text{m}$$

$$A_{st} = 11.29 \text{ cm}^2 : \frac{\phi 16 @ 170 \text{ mm}}{(1408/3.)}$$

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Mid-Span-Section

$$M_{mid} = \frac{1500 \times 3.16^2}{24} = 600 \text{ kg-m/m}$$

$$M_s = M - T \cdot x$$

$$= 600 - 1500(0.127 - 0.08)$$

$$= 498 \text{ kg-m/m}$$

$$A_{st1} = \frac{498}{f_s j d} = 3.65 \text{ cm}^2/\text{m}$$

$$A_{st2} = \frac{1500}{f_s} = 1.08 \text{ "}$$

$$A_{st} = 4.73 \text{ cm}^2/\text{m} = \frac{\phi 16 @ 340 \text{ mm}}{(10 \times 3)}$$

Design for the bottom one metre cantilevering effect of the Shoot Wall

$$M = \frac{1}{2} \times 1500 \times 1 \times \frac{1}{3}$$

$$= 416 \text{ kg-m/m}^3$$

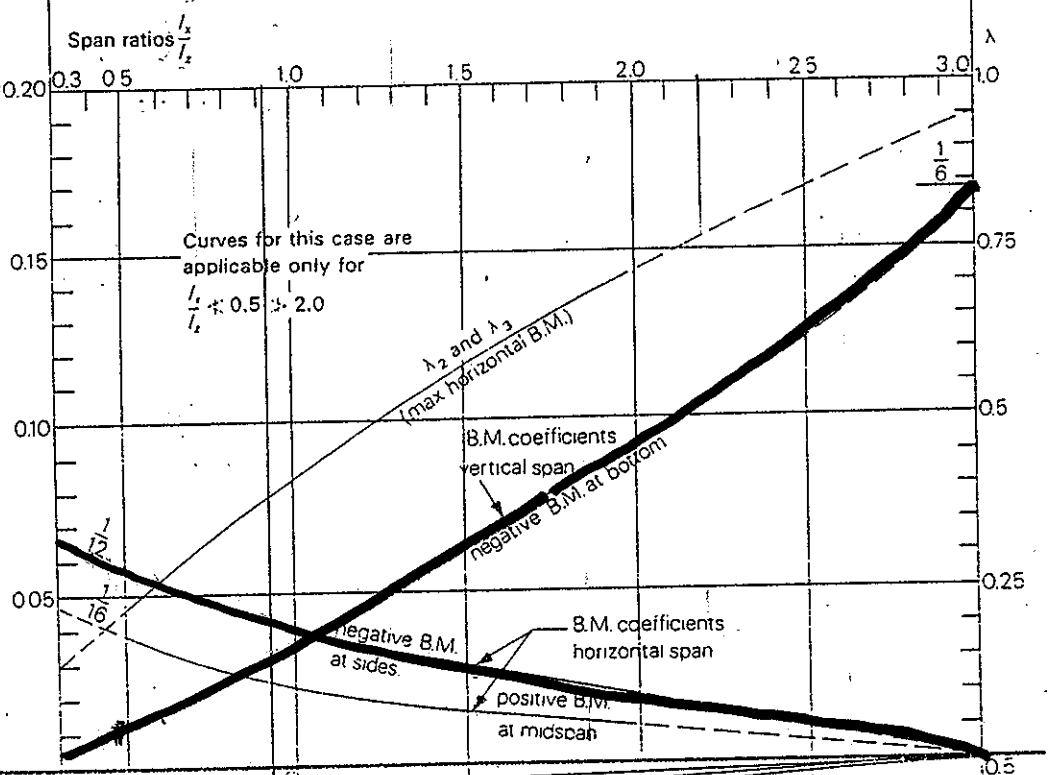
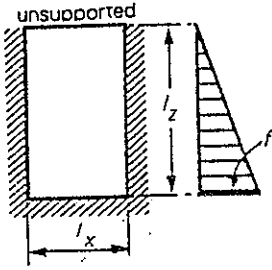
$$A_{st} = \frac{416}{f_s j d} = 4.09 \text{ cm}^2/\text{m}$$

$$A_{st_{min}} = 0.25\% = 4.00 \text{ cm}^2/\text{m} \left. \vphantom{A_{st_{min}}} \right\} \frac{\phi 10 @ 160 \text{ mm}}{(14 \times 2)}$$

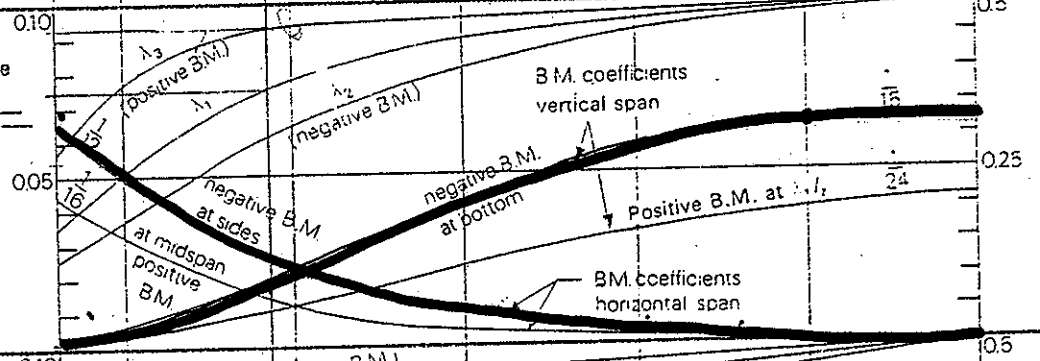
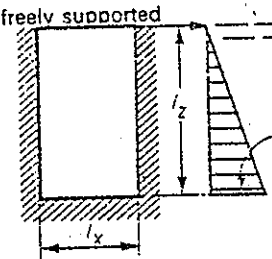
Bending-moment coefficients

bending moment = $\frac{f l^2}{12}$

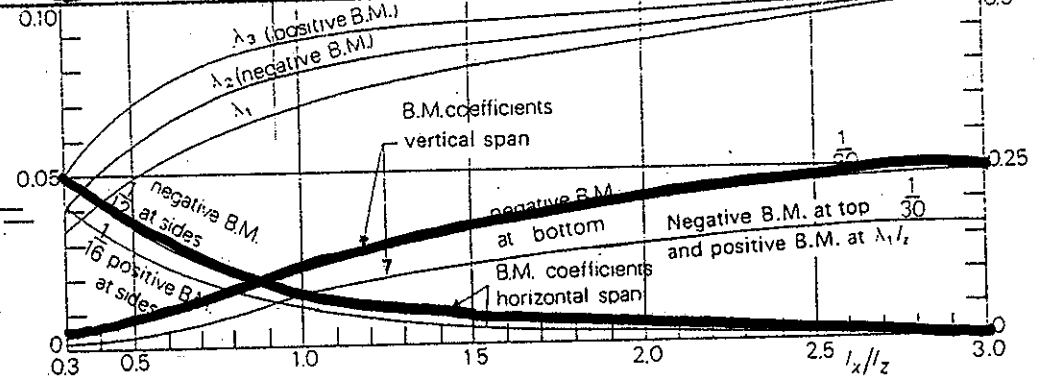
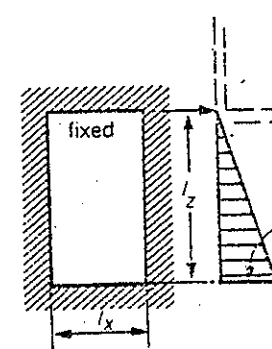
Case 1. unsupported along top edge



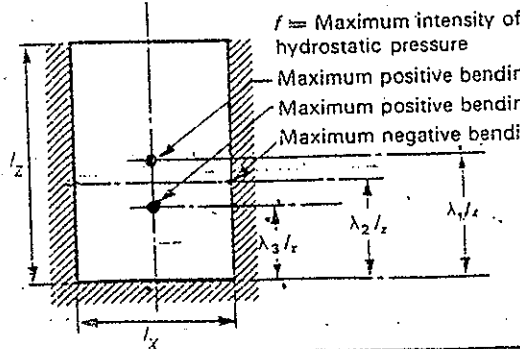
Case 2. freely supported along top edge freely supported



Case 3. Fixed or continuous along top edge



Positions of maximum bending moments



Notes

Panels fixed or continuous along bottom edge and both vertical sides; condition along top edge as indicated.

Fractions thus $\frac{1}{12}$ indicate coefficients to which curves are asymptotic or to which coefficients approach as span ratio l_x/l_z approaches zero or infinity.

Vertical span: bending moment = (coefficient) $\frac{f l_z^2}{12}$

Horizontal span: bending moment = (coefficient) $\frac{f l_x^2}{12}$

Scale on right-hand side is for values of λ_1 , λ_2 and λ_3 . Ratio of spans = $k = l_x/l_z$.