

CALCULATION SHEET

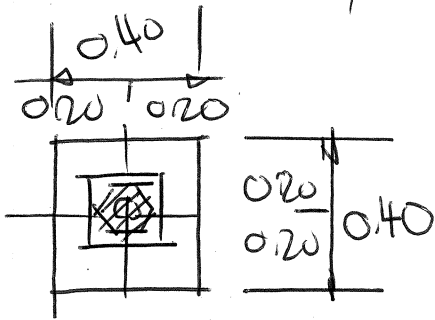
PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

#1: Max. load on footing = 2000 kg

USE 1- Pile $\phi 8" \times 6.00$

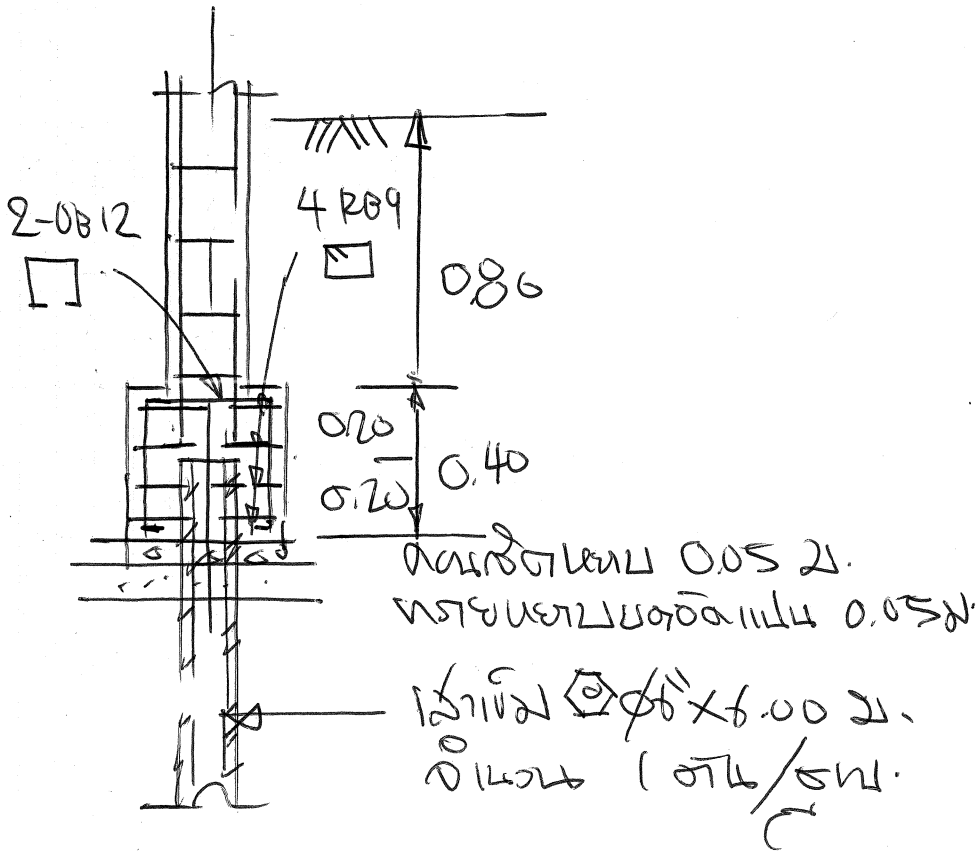
load/pile = 2000 kg

USE: Pile cap. $0.40 \times 0.40 \times 0.40$ m



$$A_{sc} = 2.88 \text{ cm}^2$$

F1: PLAN



F1: SECTION

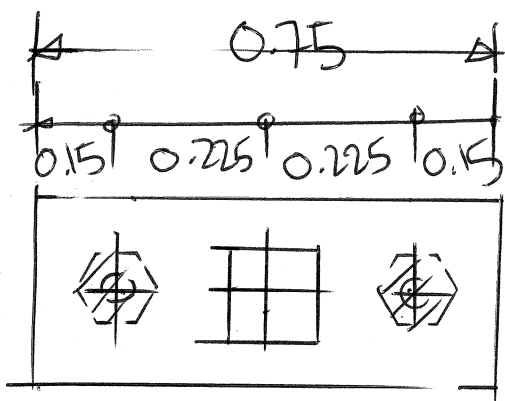
CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

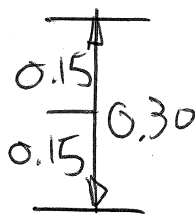
F-2 Max. load on footing = 4000 kg

USE - 2-Pile $\phi 6'' \times 6.00 \text{ m}$.

load/pile = 2000 kg



F2: PLAN



$$M = 0.15 \times 2000 = 300 \text{ kgm}$$

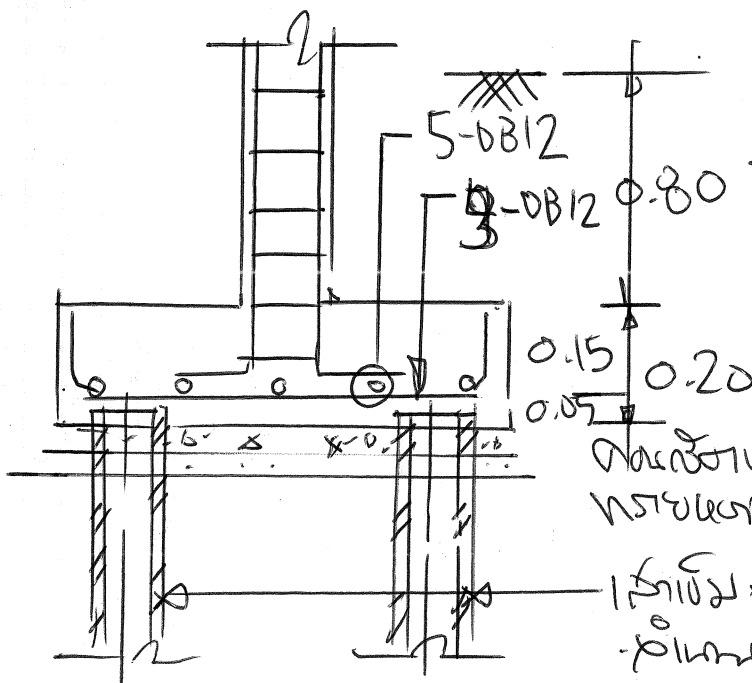
$$A_{st} = 1.30 \text{ cm}^2 \quad \left. \begin{array}{l} \\ \\ \end{array} \right\} 3 \text{ DB12}$$

$$\Sigma_o = 6.73 \text{ cm}$$

$$b_f = 2.22 \text{ ksc}$$

$$b_p = 4.45 \text{ ksc}$$

$$A_{st} = 2.7 \text{ cm}^2 = 5 \text{ DB12}$$



F2: SECTION

0.05 ၁.
 ၀.၀၅၁
 ၁.၅၀၁ $\phi 6'' \times 6.00$ ၁.
 ၂ ခု ၂ ခု/၅၇၄.

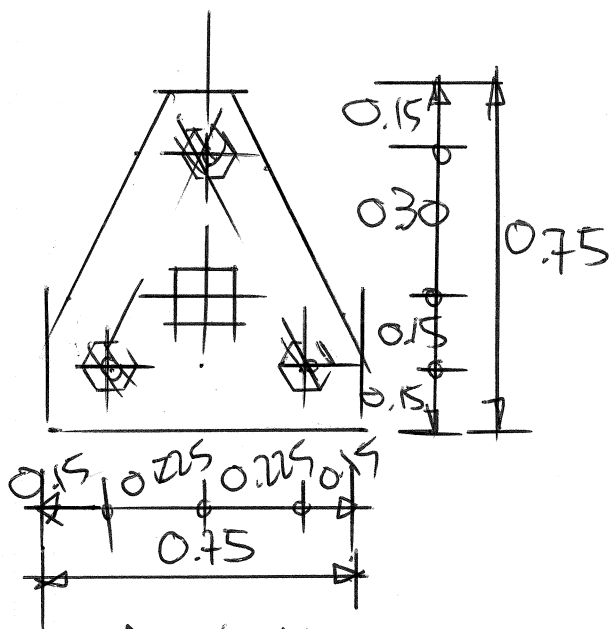
CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

F3. Max. load on footing = 6000 kg

USE: 3-Pile @ $\phi 6'' \times 6.00$ m.

load/pile = 2000 kg



F3: PLAN

$$M = 2000 \times 0.225 = 450 \text{ kg-m}$$

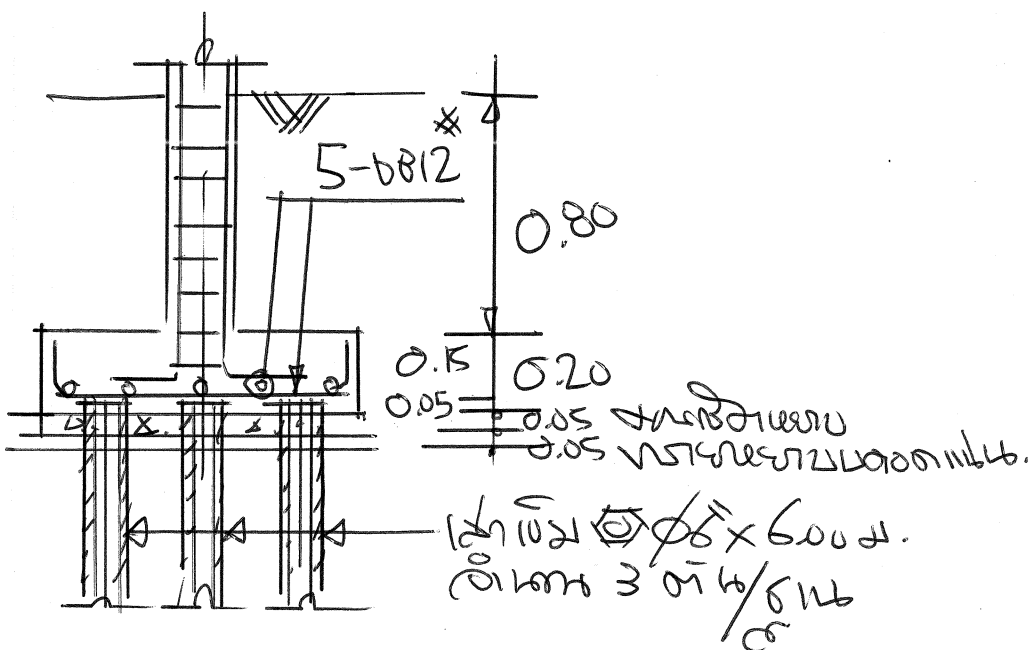
$$A_s = 1.95 \text{ cm}^2$$

$$\Sigma A_s = 6.73 \text{ cm}^2 \quad \left. \begin{array}{l} \\ \\ \end{array} \right\} 5-0812$$

$$A_{smin} = 2.70 \text{ cm}^2 \quad 5-0812$$

$$l_d = 0.83 \text{ ksc}$$

$$l_p = 3.34 \text{ ksc}$$



F3: SECTION.

CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

F-4 : Max load on footing = 18000 kg
USE = 4-Pile @ $\phi 6 \times 6.00$ m.
load/pile = 2000 kg

$$\begin{aligned} \Delta I &= 2 \times 2000 \times 0.15 \\ &= 600 \text{ kg.m} \end{aligned}$$

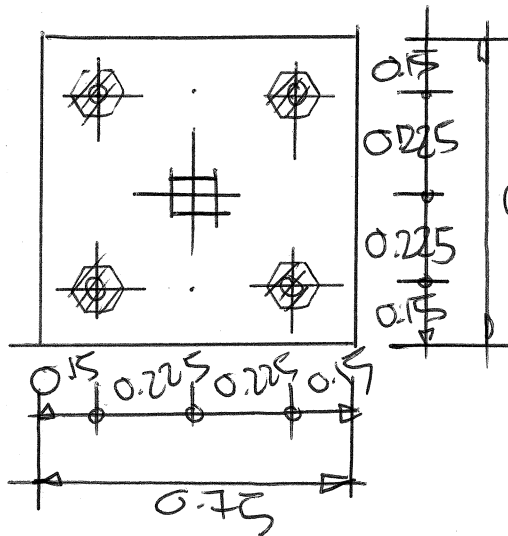
$$A_{fy} = 2.59 \text{ cm}^2 \quad \left. \begin{array}{l} \\ \\ \end{array} \right\} 5-0812$$

$$\Sigma \bar{y} = 13.47 \text{ cm}$$

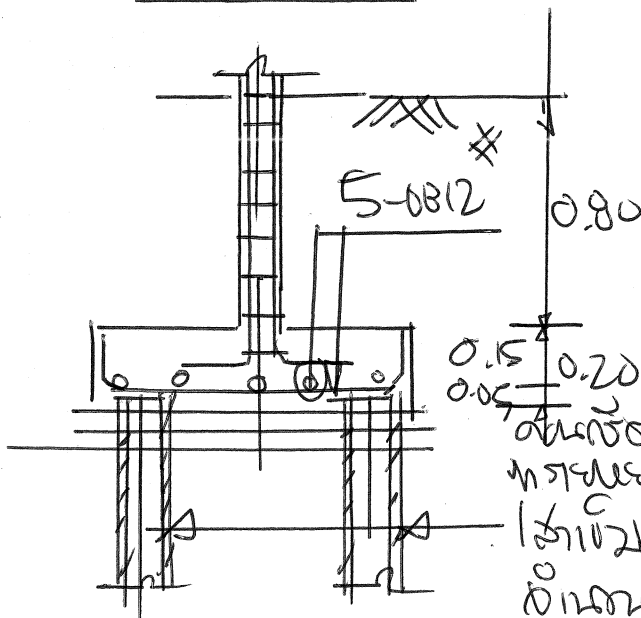
$$I_{stmin} = 2.70 \text{ cm}^2$$

$$b_f = 1.78 \text{ ksc}$$

$$b_p = 4.45 \text{ ksc}$$



F4: PLAN



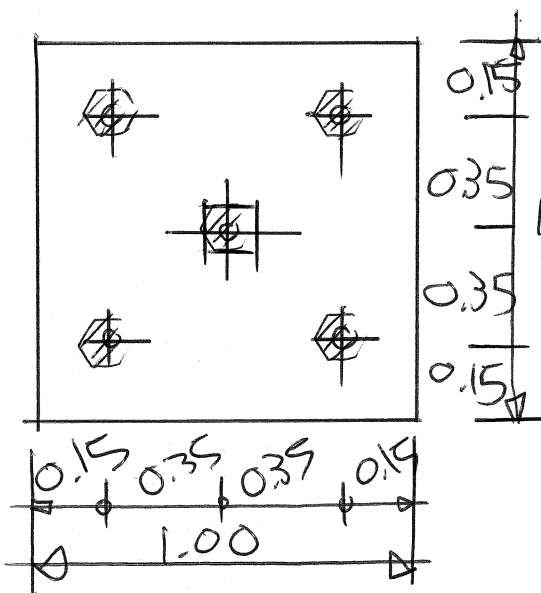
F4: SECTION

အပေါ်ဘက် 0.05၂.
 အောက်ဘက် 0.05၂.
 ခံနိုင်ရည် $\phi 6 \times 6.00$ ၂.
 ပုံအရ 4-014/516.
 C

CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

F-5 : Max. load on Footing = 70000 kg
USE : 5-Pile @ $\phi 6'' \times 6.00 \text{ m}$
load/Pile = 2000 kg/Pile



$$M = (2) \times 2000 \times 0.275 = 1100 \text{ kgm}$$

$$A_f = 4.75 \text{ cm}^2$$

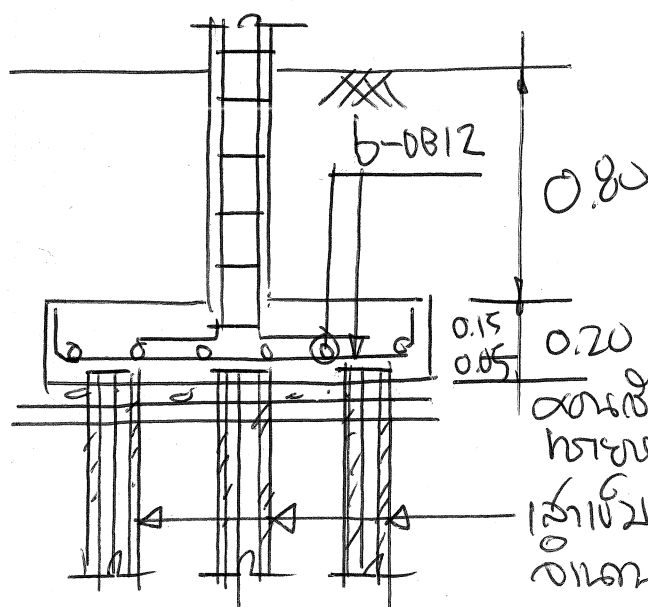
$$\Sigma \phi = 13.47 \text{ cm}$$

$$b_f = 2.45 \text{ ksc}$$

$$G_p = 5.56 \text{ ksc}$$

} b-0812

F5: PLAN



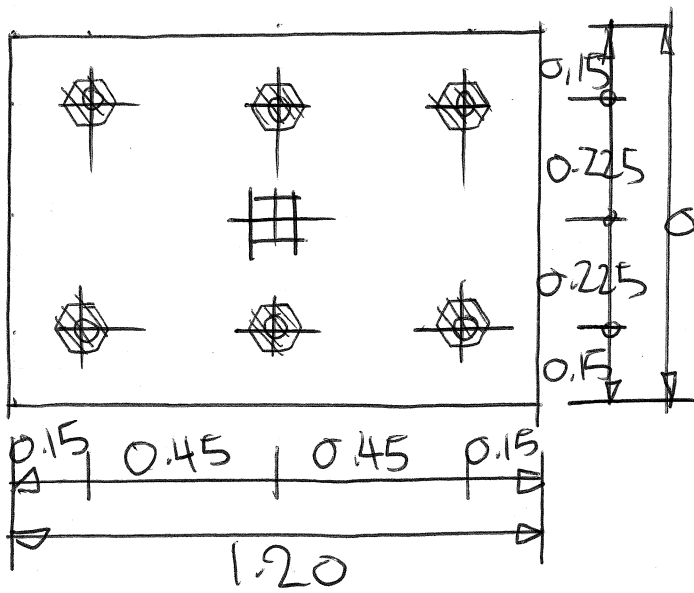
အားပေးပုံအား 0.05 ခု
 ကာကွယ်ပုံ 0.05 ခု
 ၁၅၀၀၂ @ $\phi 6'' \times 6.00 \text{ m}$
 ဝါး ၅ ဝါး/ပုံ

F5: SECTION

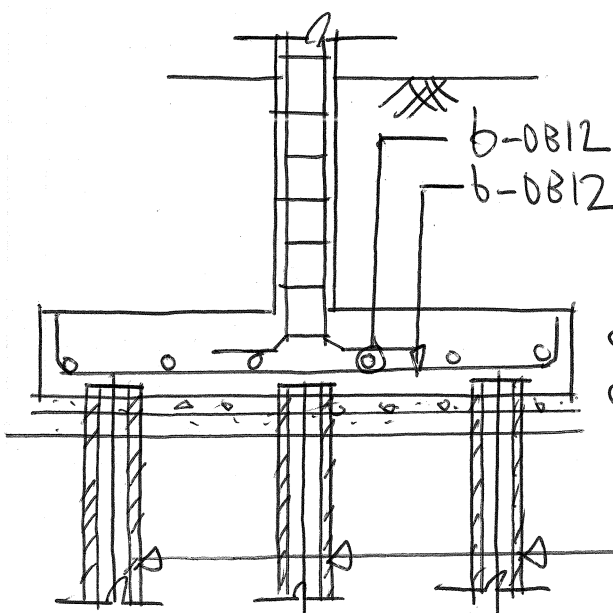
CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

$F_b = \text{Max. load on footing} = 12000 \text{ kg}$
 $USE = \text{b-pile } \odot \phi 6'' \times 6.00 \text{ m.}$
 $\text{load/pile} = 2000 \text{ kg}$



F6: PLAN



F6: SECTION

$M_s = 900 \text{ kg.m}$
 $A_{sf} = 3.89 \text{ cm}^2$
 $\Sigma_o = 20.2 \text{ cm}^2$ } b-0812

$M_u = 1500 \text{ kg.m}$
 $A_{fu} = 6.48 \text{ cm}^2$
 $\Sigma_o = 13.46 \text{ cm}^2$ } b-0812
 $A_{stmin} = 4.32 \text{ cm}^2$

$b_f = 1.67 \text{ ksc}$

$b_{f_s} = 3.56 \text{ ksc}$

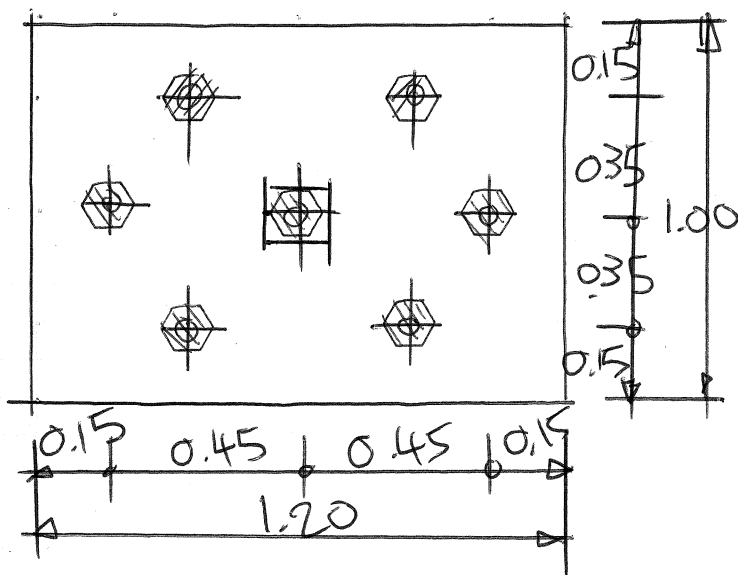
$b_p = 6.67 \text{ ksc}$

0.052
 0.052
 $\odot \phi 6'' \times 6.00$
 $b \ 074/074$

CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

F-7: Max. load on Footing = 14000 kg
USE = 7-Pile $\odot \phi 6 \times 6.00$ m.
load/Pile = 2000 kg



F7: PLAN

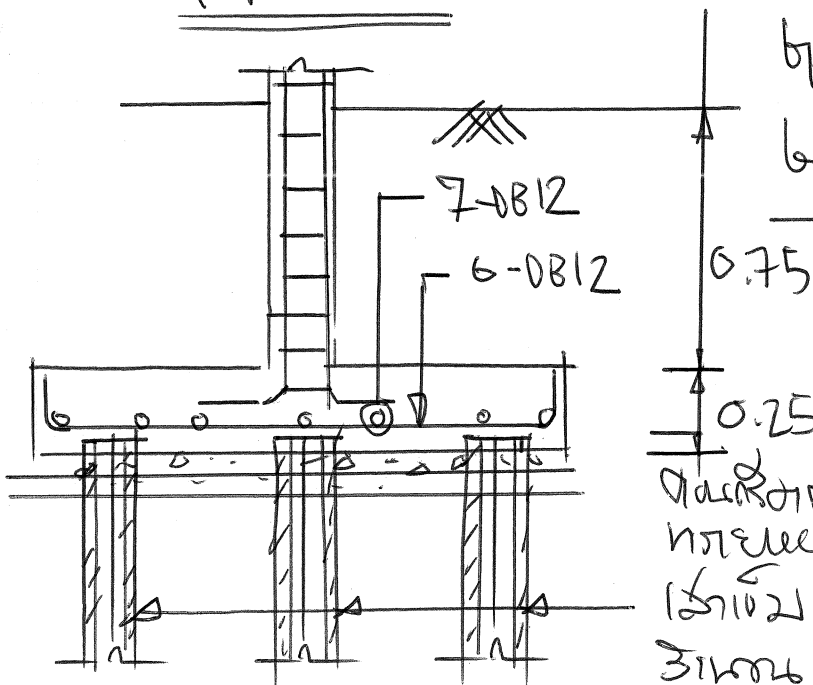
$M_S = 1100 \text{ kg.m}$
 $A_{f_s} = 3.56 \text{ cm}^2$
 $\Sigma \phi = 10.1 \text{ cm}$ } 7-0812

$M_L = 1350 \text{ kg.m}$
 $A_{f_l} = 4.37 \text{ cm}^2$
 $\Sigma \phi = 15.2 \text{ cm}$ } 6-0812

$b_{f_s} = 0.45 \text{ m}$

$b_{f_l} = 2.00 \text{ m}$

$b_{op} = 5.00 \text{ msc.}$



F7: SECTION

အောက်ဖျား ၀.၀၅၂.
 အပေါ်ဖျား ၀.၀၅၂.
 အကျယ် $\odot \phi 6 \times 6.00$ m.
 အား ၇ ဝါး/ဝါး.

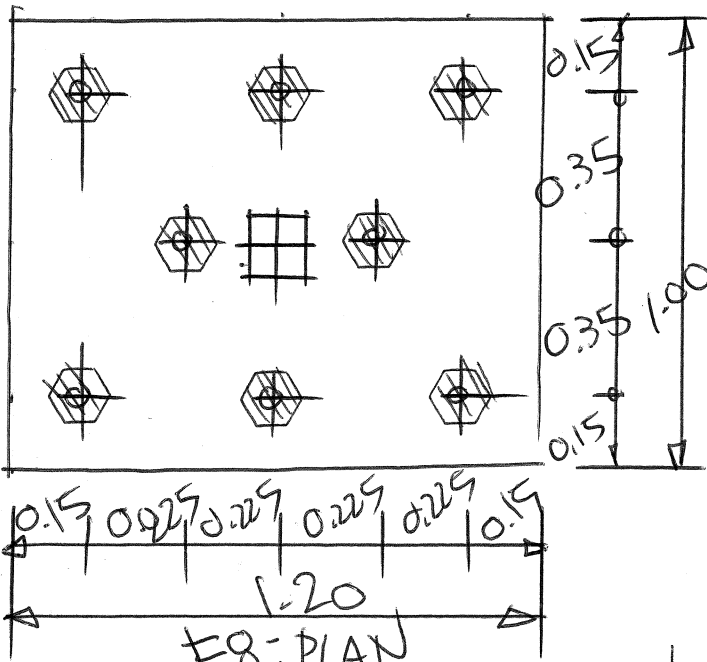
CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

F.8 : Max load on footing = 16000 kg

USE: 8-Pile $\phi 6'' \times 6.00$ m.

load/pile = 2000 kg/pile



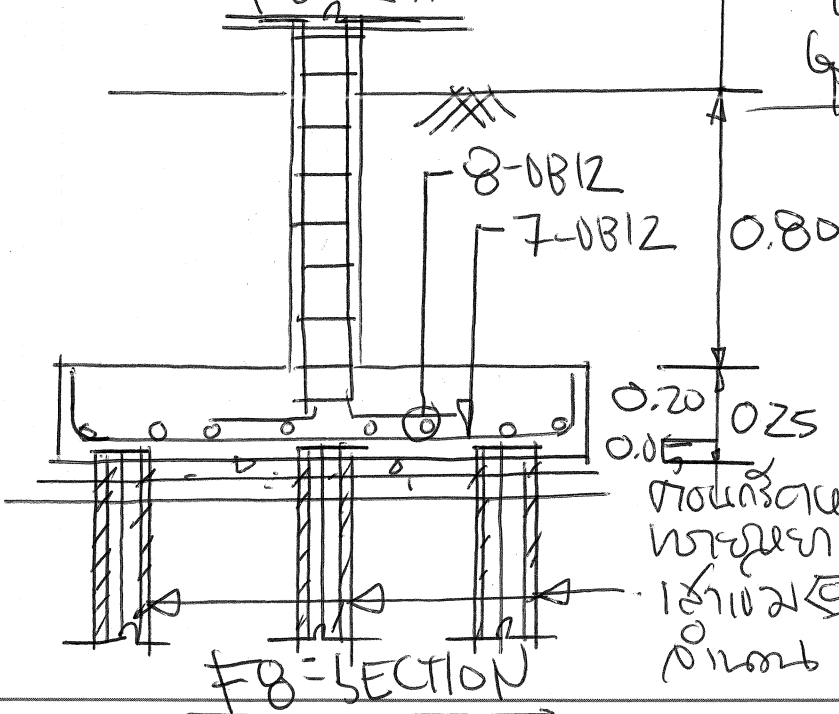
$M_k = 1650 \text{ kgm}$
 $A_b = 5.34 \text{ cm}^2$
 $\Sigma \sigma = 15.2 \text{ cm}$ } 7-0812

$M_L = 1800 \text{ kgm}$
 $A_b = 5.83 \text{ cm}^2$
 $\Sigma \sigma = 15.2 \text{ cm}$ } 8-0812

$g_s = 2.30 \text{ ksc}$

$g_L = 2.50 \text{ ksc}$

$g_p = 5.71 \text{ ksc}$



1. 1.50m x 0.05m
 2. 1.50m x 0.05m
 3. 1.50m x 0.05m
 4. 1.50m x 0.05m
 5. 1.50m x 0.05m
 6. 1.50m x 0.05m
 7. 1.50m x 0.05m
 8. 1.50m x 0.05m

F8-SECTION

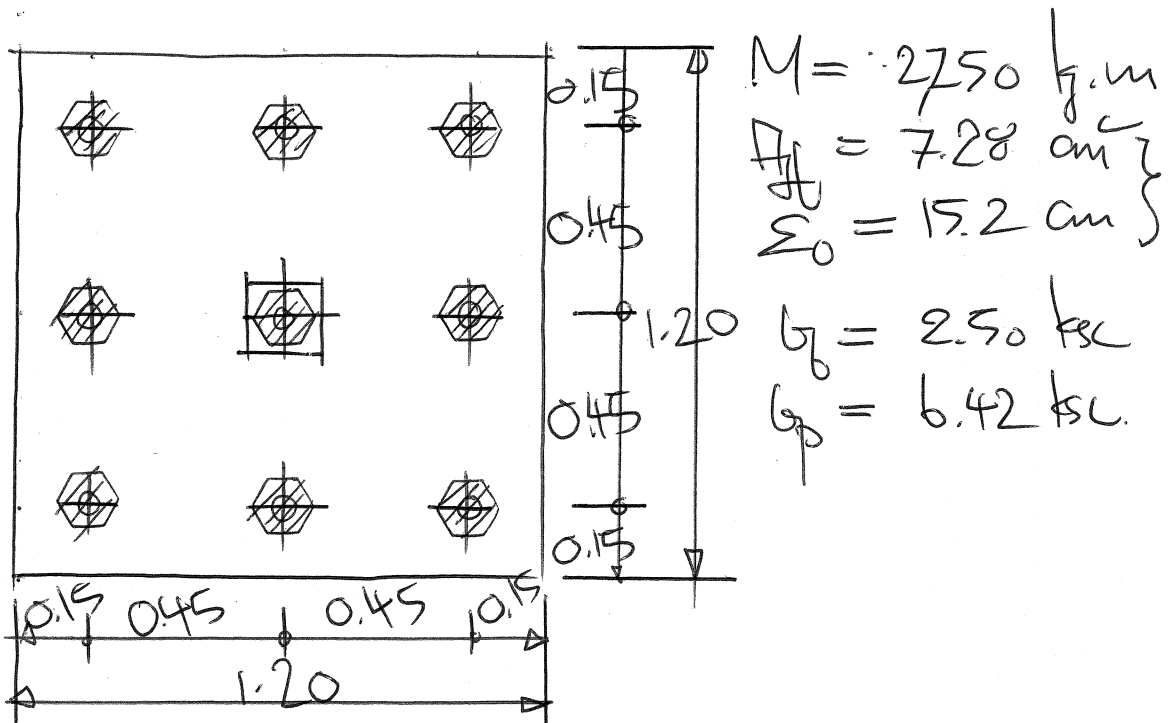
CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

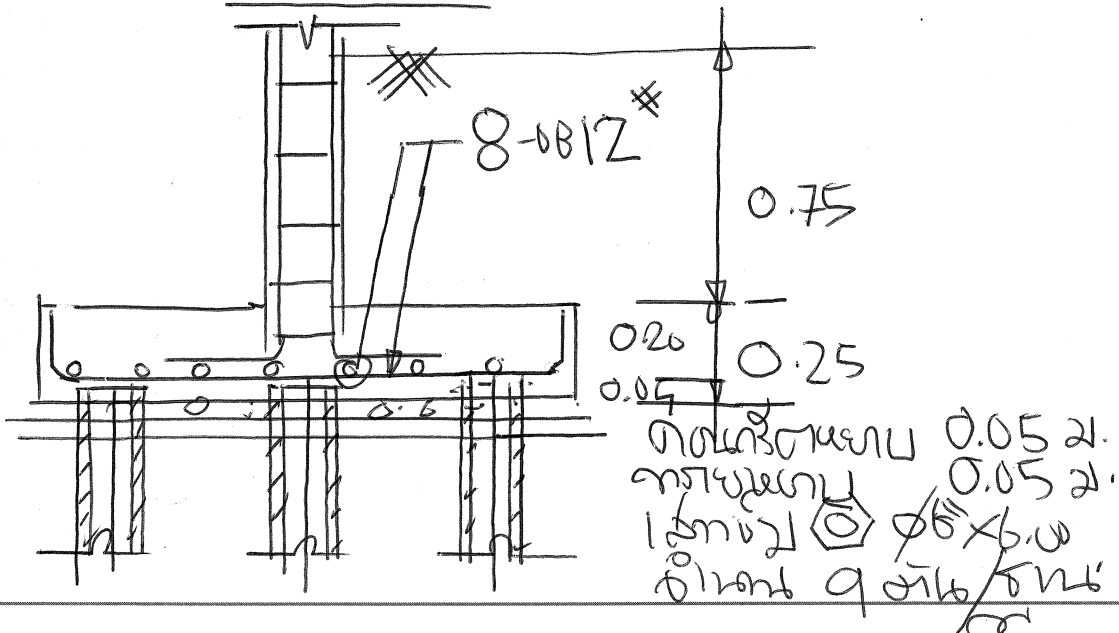
Eq: Max. load on footing = 78,000 kg

USE = 9-Pile $\phi 6'' \times 6.00$

load/pile = 2000 kg/pile



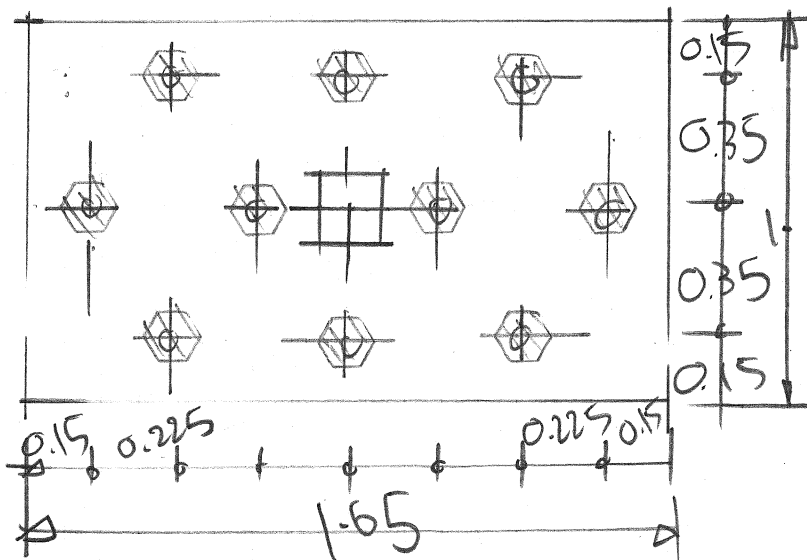
Eq: PLAN



CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

F10 : Max load on footing = 20000 kg
USE : 10-Pile $\phi 6'' \times 6.00'$
load/pile = 2000 kg



F10: PLAN

$$M_s = 1650 \text{ kgm}$$

$$A_{fb} = 4.28 \text{ cm}^2$$

$$\Sigma \sigma_0 = 12.12 \text{ cm}$$

$$A_{tmin} = 8.91 \text{ cm}^2$$

9-DB12

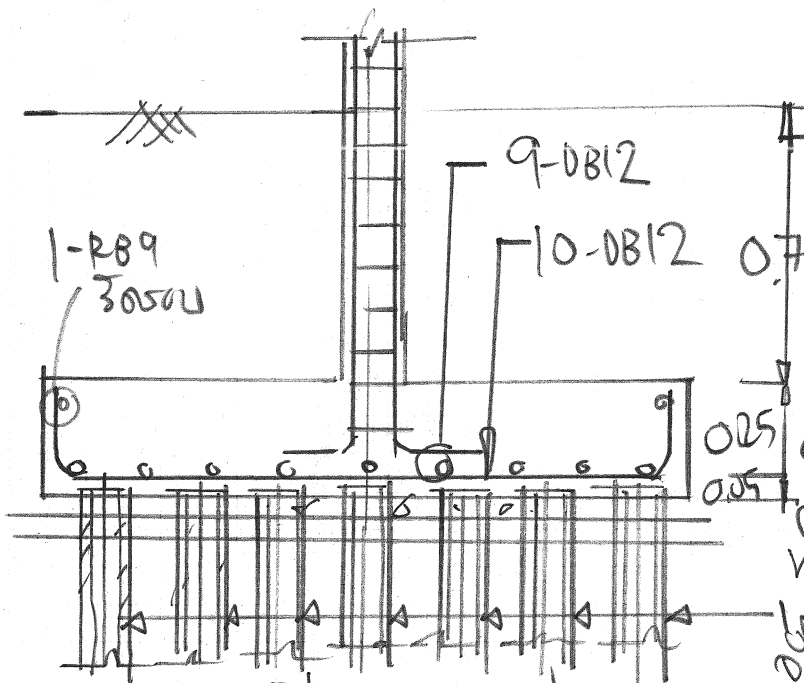
$$M_L = 3000 \text{ kgm}$$

$$A_{tL} = 7.76 \text{ cm}^2$$

$$\Sigma \sigma_0 = 16.16 \text{ cm}$$

$$A_{tmin} = 5.4 \text{ cm}^2$$

10-DB12



F10-SECTION

$$b_b = 0.85 \text{ KSC}$$

$$b_{bL} = 2.53 \text{ KSC}$$

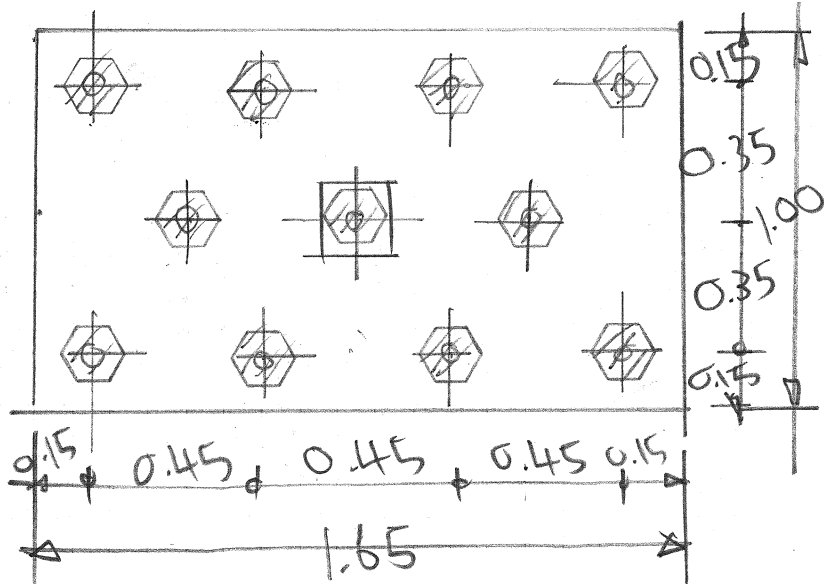
$$b_p = 2.00 \text{ KSC}$$

0.05 2.
 0.05 2.
 $\phi 6'' \times 6.00'$
 10 piles / sheet

CALCULATION SHEET

PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

F11: Max. load on Footing = 22,000 kg
USE 11 - pile $\phi 6'' \times 6.00$
load/pile = 2000 kg

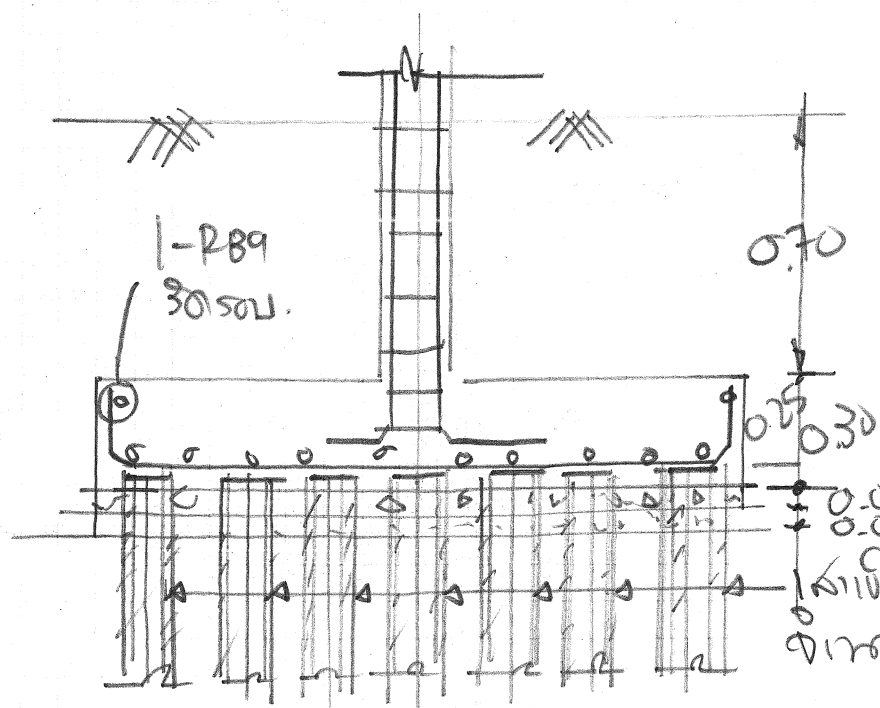


F11: PLAN

$M_s = 2200 \text{ kgm}$
 $A_f = 5.7 \text{ cm}^2$
 $Z_o = 16.2 \text{ cm}$
 $A_{fm} = 8.91 \text{ cm}^2$
 10-08-12

$M_L = 3750 \text{ kgm}$
 $A_f = 9.71 \text{ cm}^2$
 $Z_o = 20.2 \text{ cm}$
 12-03-12

$V_f = 2.60 \text{ Ksc}$
 $b_p = 5.50 \text{ Ksc}$



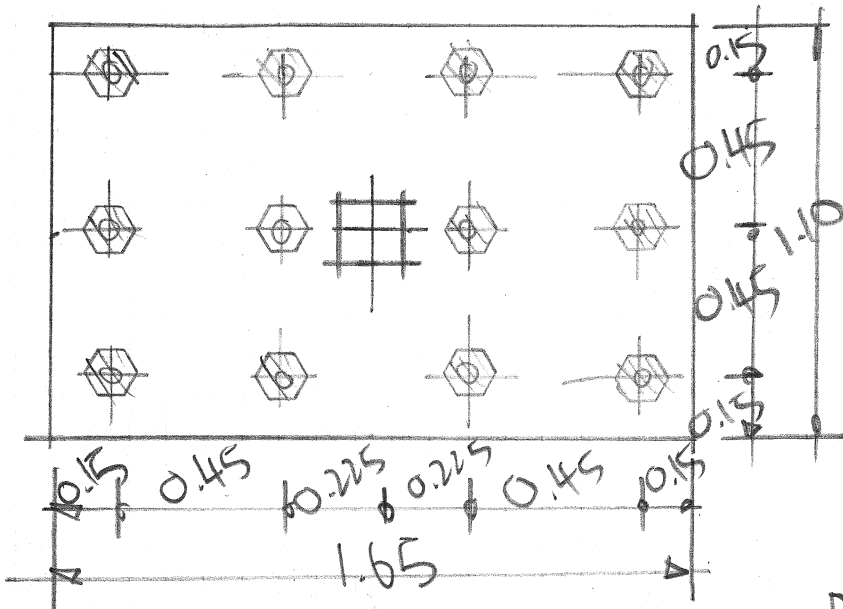
F11: SECTION

1-R89
 30
 $\phi 6'' \times 6.00$
 11
 0.05
 0.05

CALCULATION SHEET

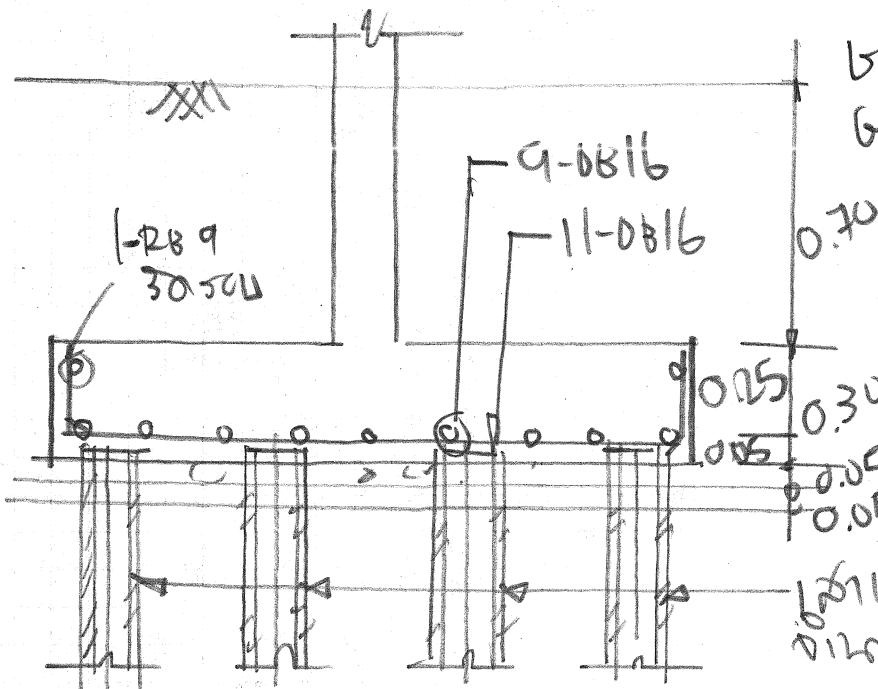
PROJECT	JOB ID	
SUBJECT	DESIGNED	PAGE
	CHECKED	SHEET

F12 = Max. load on Footing = 24000 kg
USE 12-Pile @ $\phi 6'' \times 6.00$
load/Pile = 2000 kg



F12: PLAN

$M_s = 3000 \text{ kg}\cdot\text{m}$
 $A_{st} = 7.77 \text{ cm}^2$
 $\Sigma_0 = 16.2 \text{ cm}$
 $A_{st_{min}} = 8.91 \text{ cm}^2$ 9-DB16
 $M_L = 4500 \text{ kg}\cdot\text{m}$
 $A_{st} = 11.65 \text{ cm}^2$
 $\Sigma_0 = 24.25 \text{ cm}$
 $A_{st_{min}} = 5.94 \text{ cm}^2$ 11-DB16



F12: SECTION

$b_f = 3.77 \text{ krc}$
 $g_f = 6.00 \text{ krc}$

12-DB16 @ $\phi 6'' \times 6.00$
 12-DB16/514
 c'