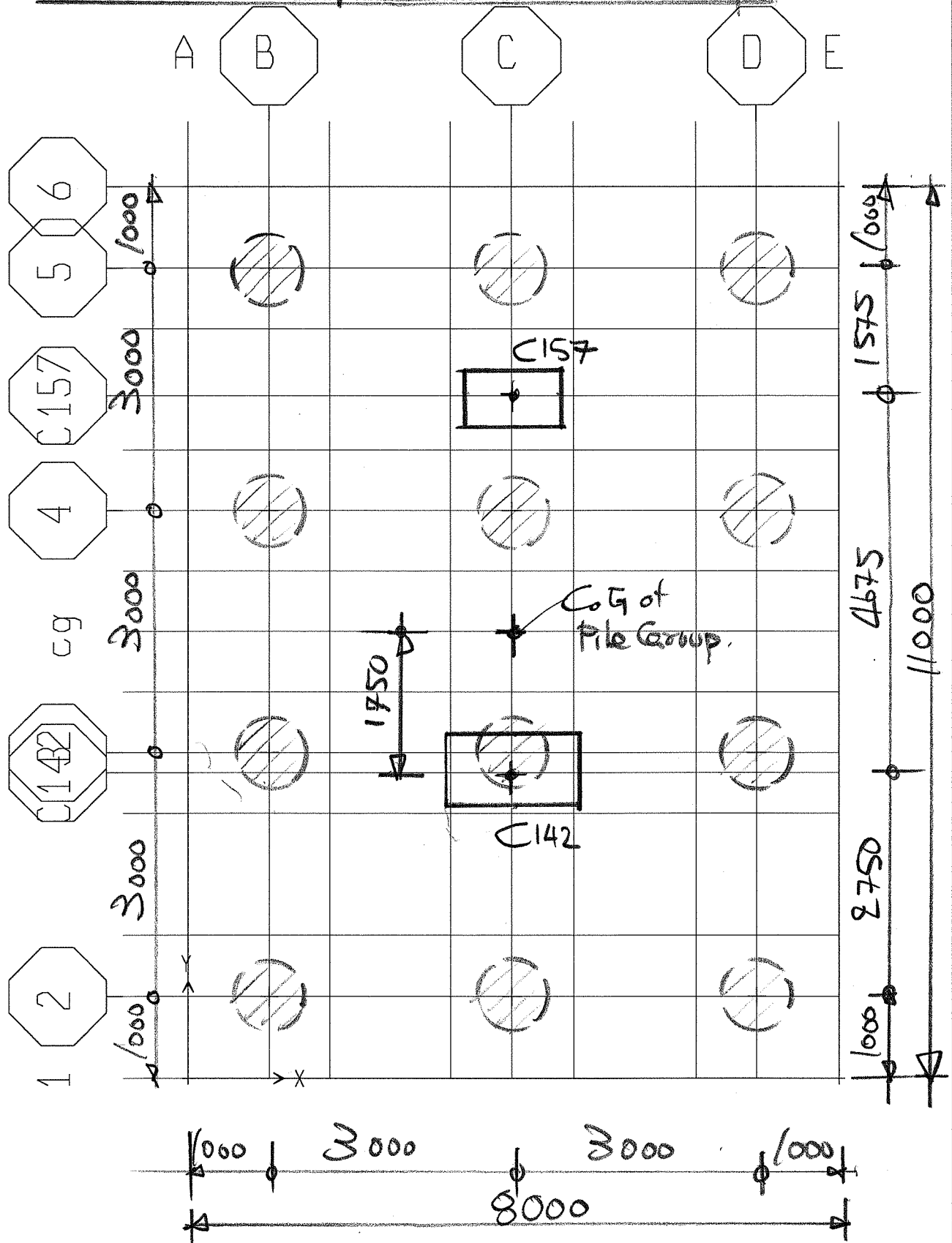


# Design of 12 F100

Provided Pile cap: 8.00 x 11.00 x 2.50dp.



12-F100: PLAN

# Loading Form Analysis

Reaction : 12-F100

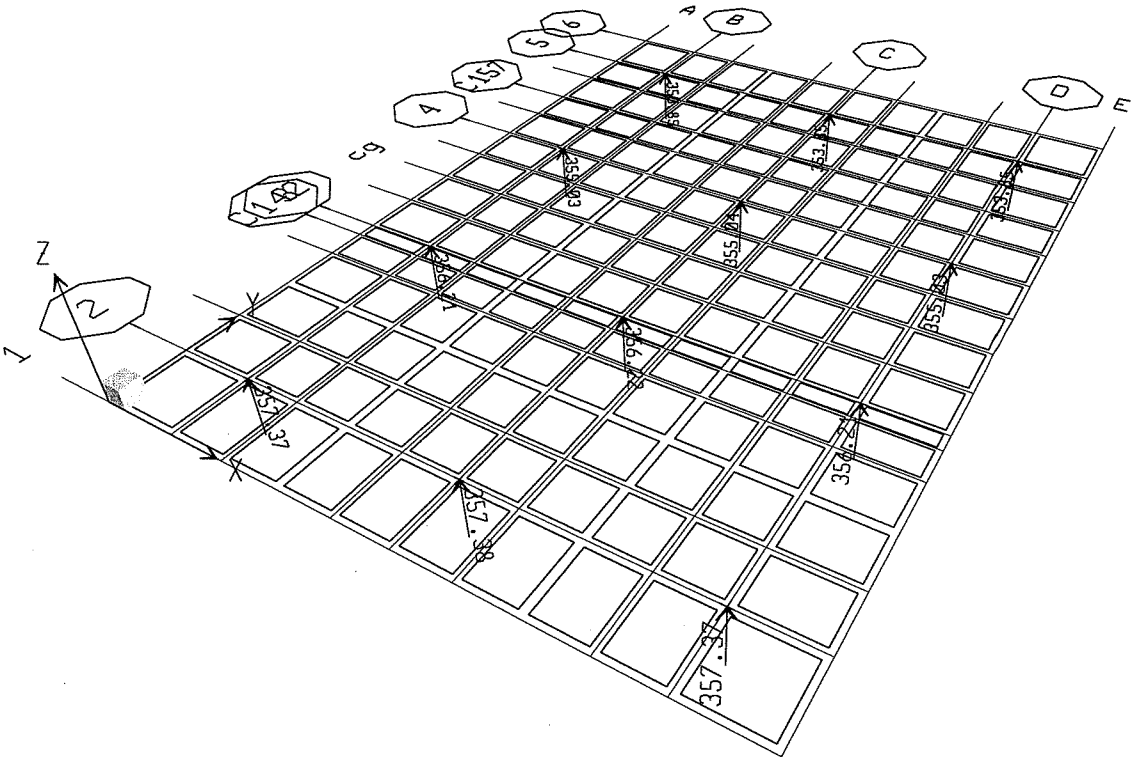
Point Labe	DL [ton]	LL [ton]	DL+LL [ton]	Lx [m]	Lx*[DL+LL]	Ly [m]	y*[DL+LL]
C157	1,099	329	1,428	0.00	0	4.68	6,676
C142	1,859	558	2,417	0.00	0	0.00	0
$\Sigma =$	2,958	887	3,845		0		6,676

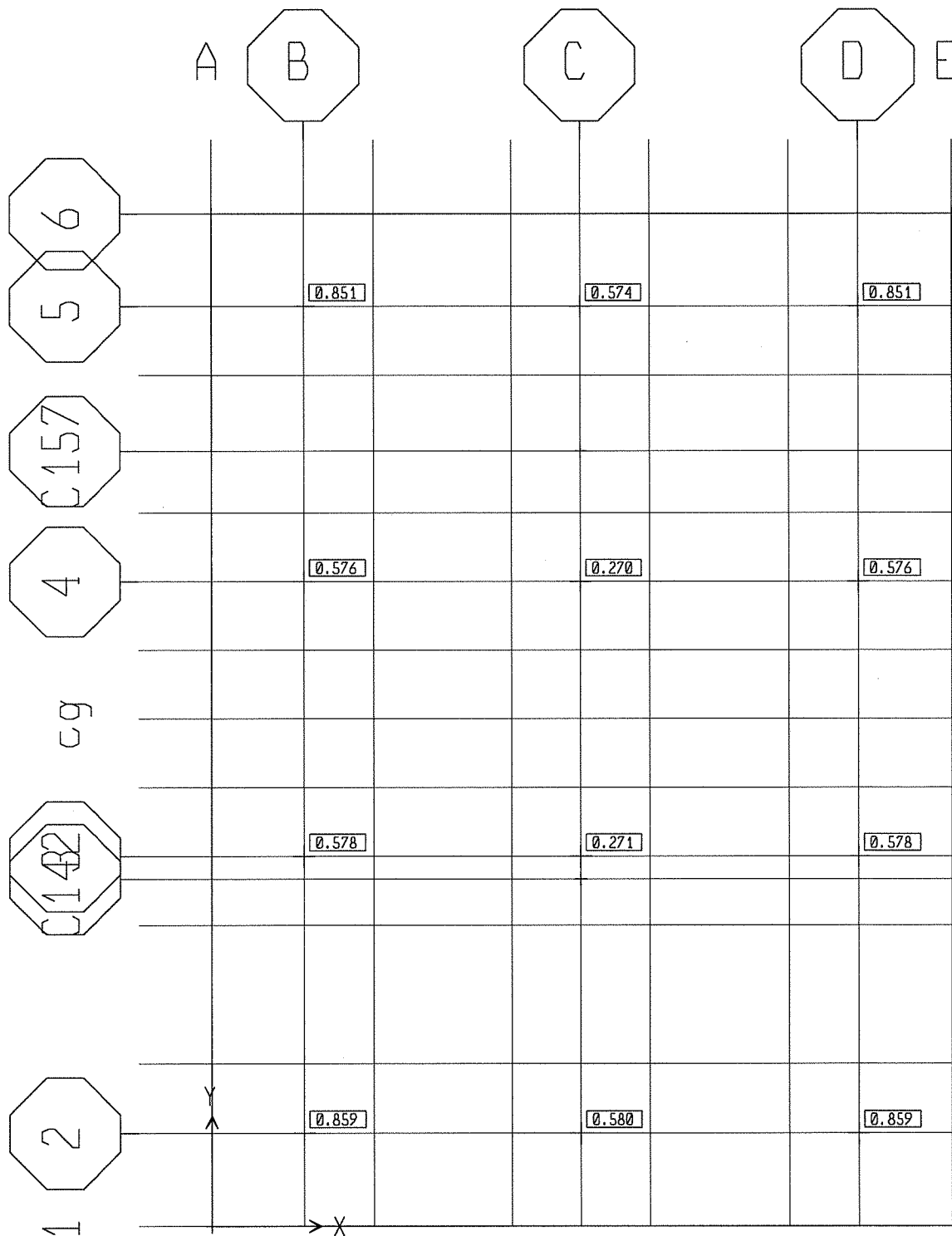
Lx avg = 0.00 Ly avg = 1.74

Provide Bore Pile Dia. 1000 mm. = 12  
SW : 8.00x11.00x2.5x2.4 = 528 ton  
Load / Pile = 364 ton

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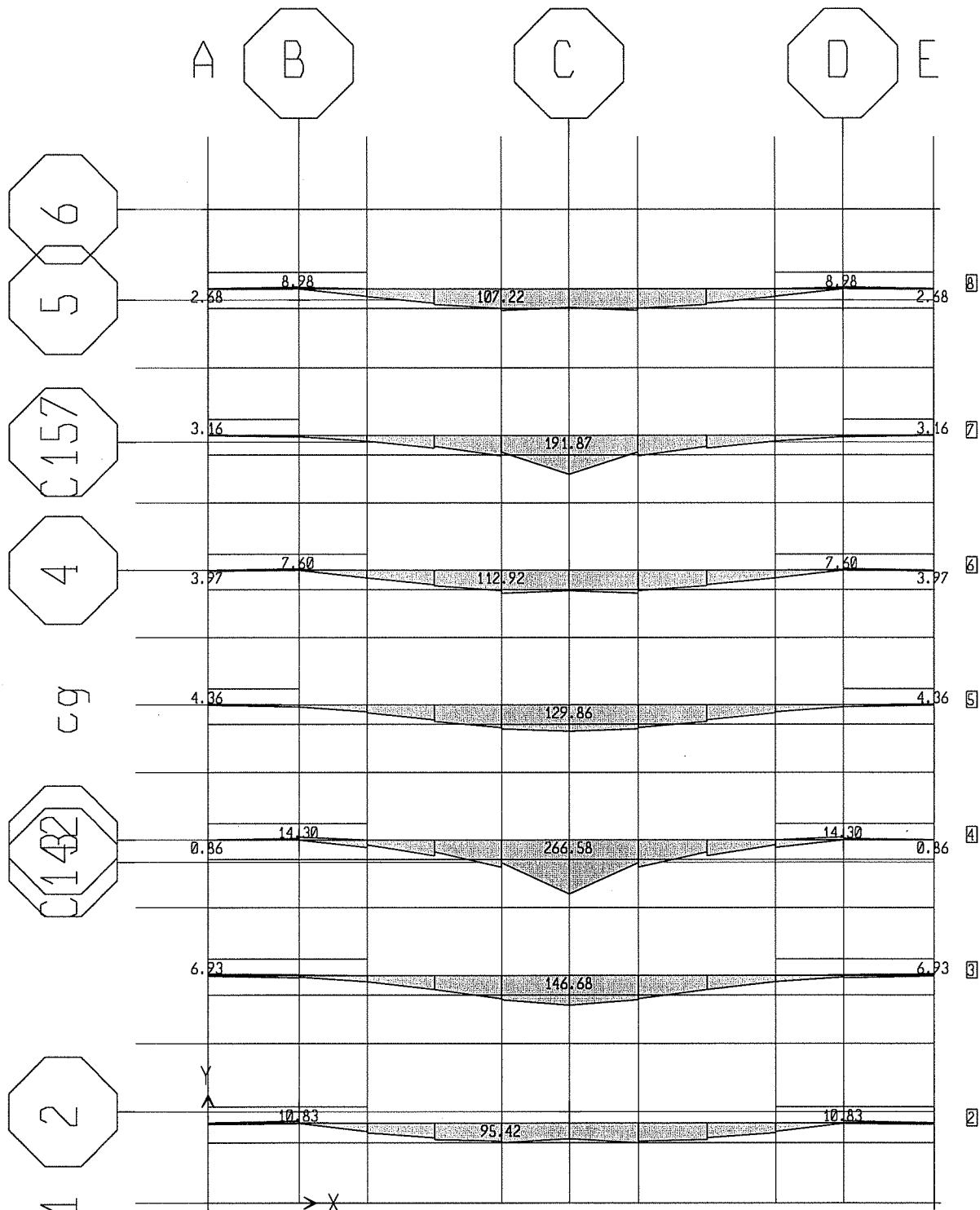
# Joint Reaction





Punching Shear Capacity Ratios

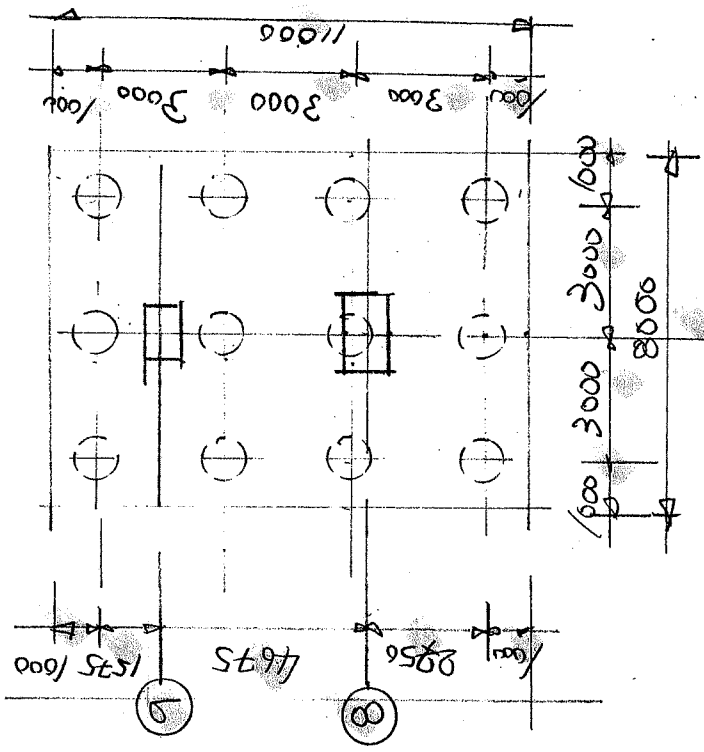
# X-Strip Reinforcement



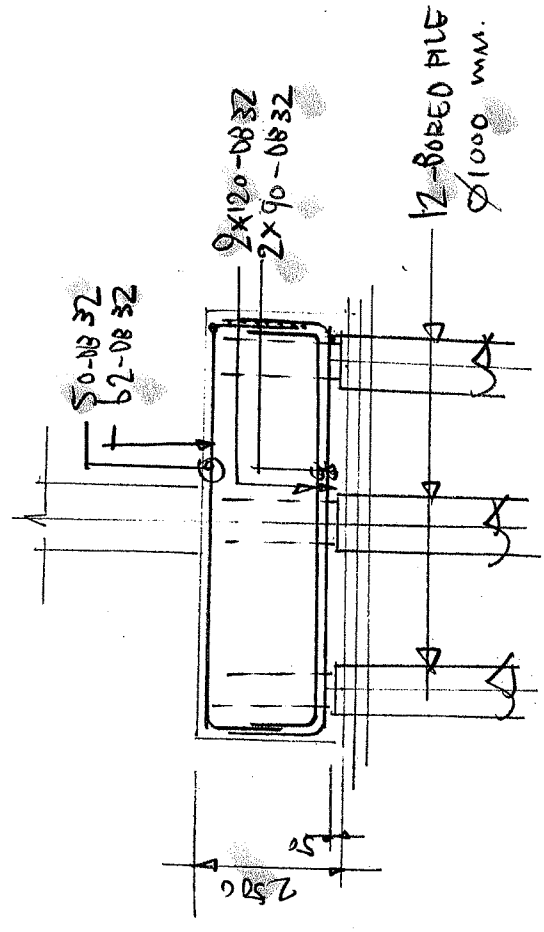
TOP Reinf :  $A_{strip} = 1130 \times 7 = 100.1 \text{ cm}^2$   
 $A_{min} = 0.0015 \times 1100 \times 250 = 413 \text{ cm}^2$  } = 62-08 32 (T)

Bot. Reinf :  $A_{strip} = 266.617 = 1867 \text{ cm}^2 = \underline{2 \times 120-08 32 (B)}$

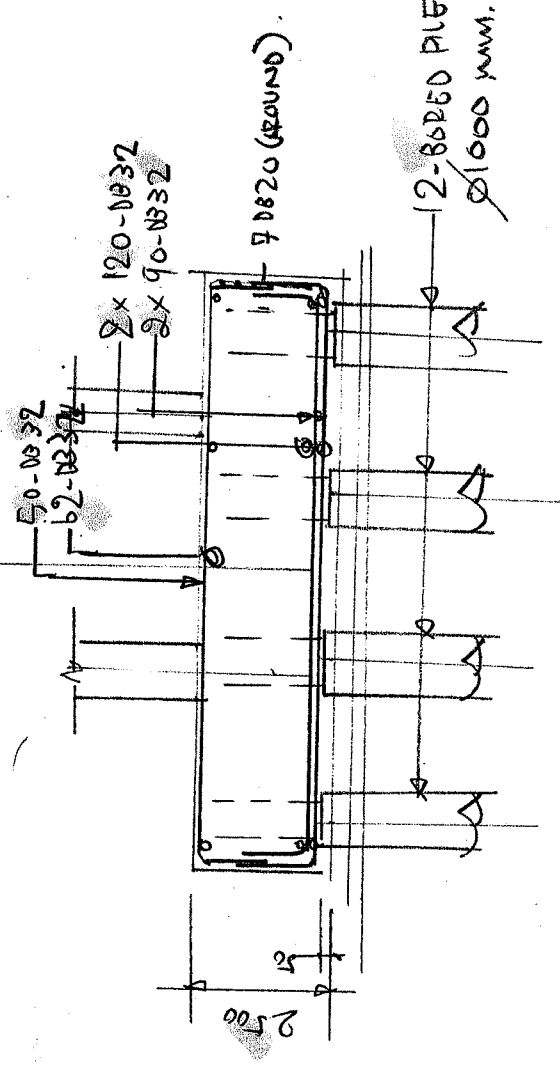




12-F100 = PLAN



12-F100 : SECTION 1-1

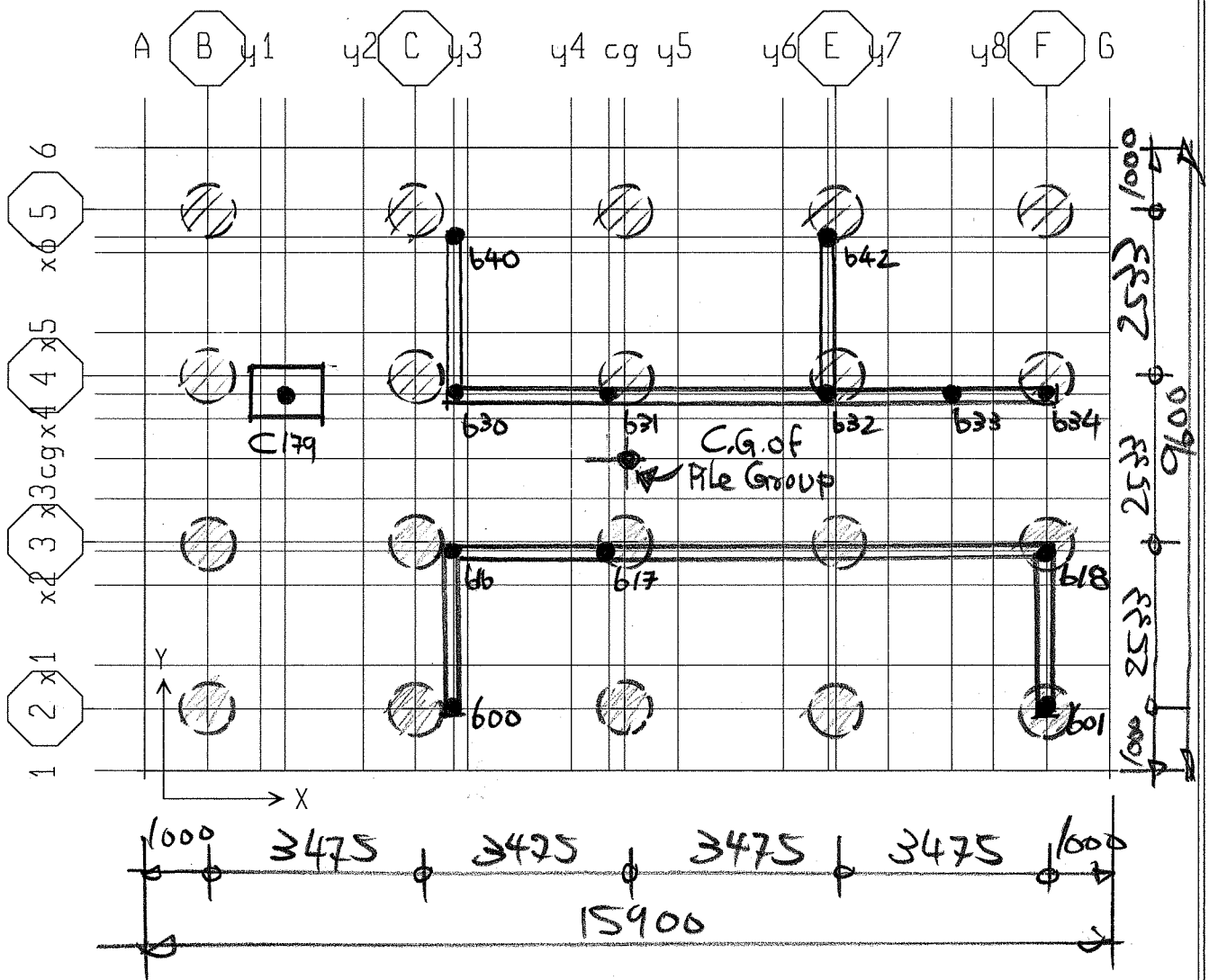


12-F100 : SECTION 2-2

Y9100

# Design of 20-F100

Provided Pile Cap: 15.90 x 9.60 x 2.50 dp



20-F100: PLAN

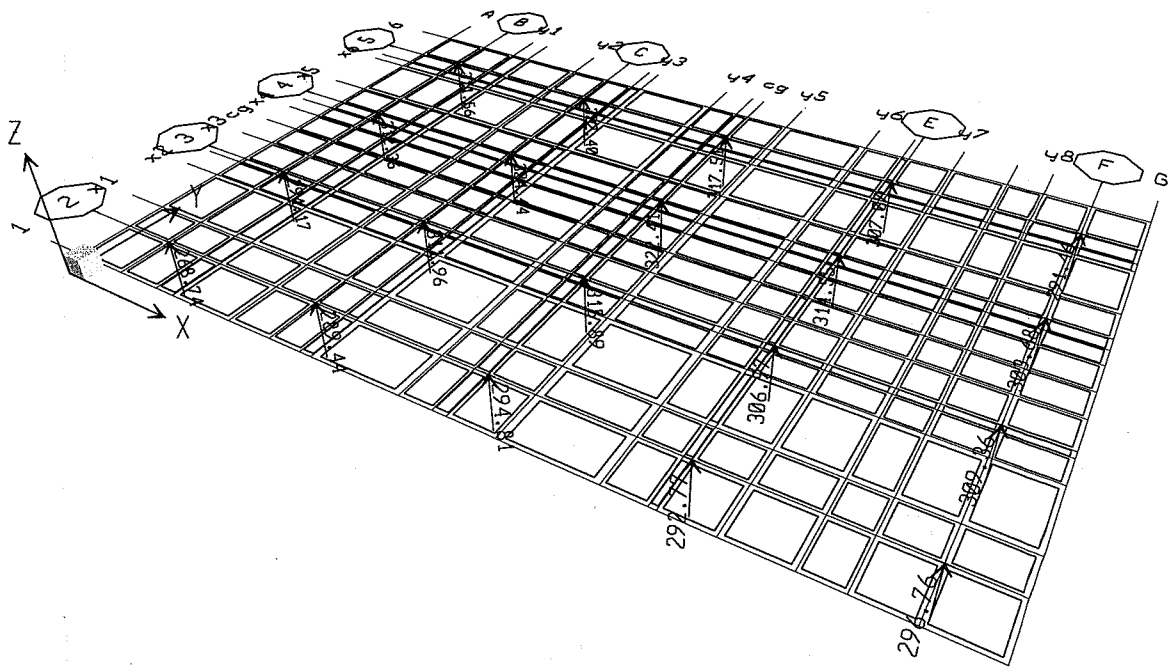


# Loading Form Analysis

Reaction : 20-F100

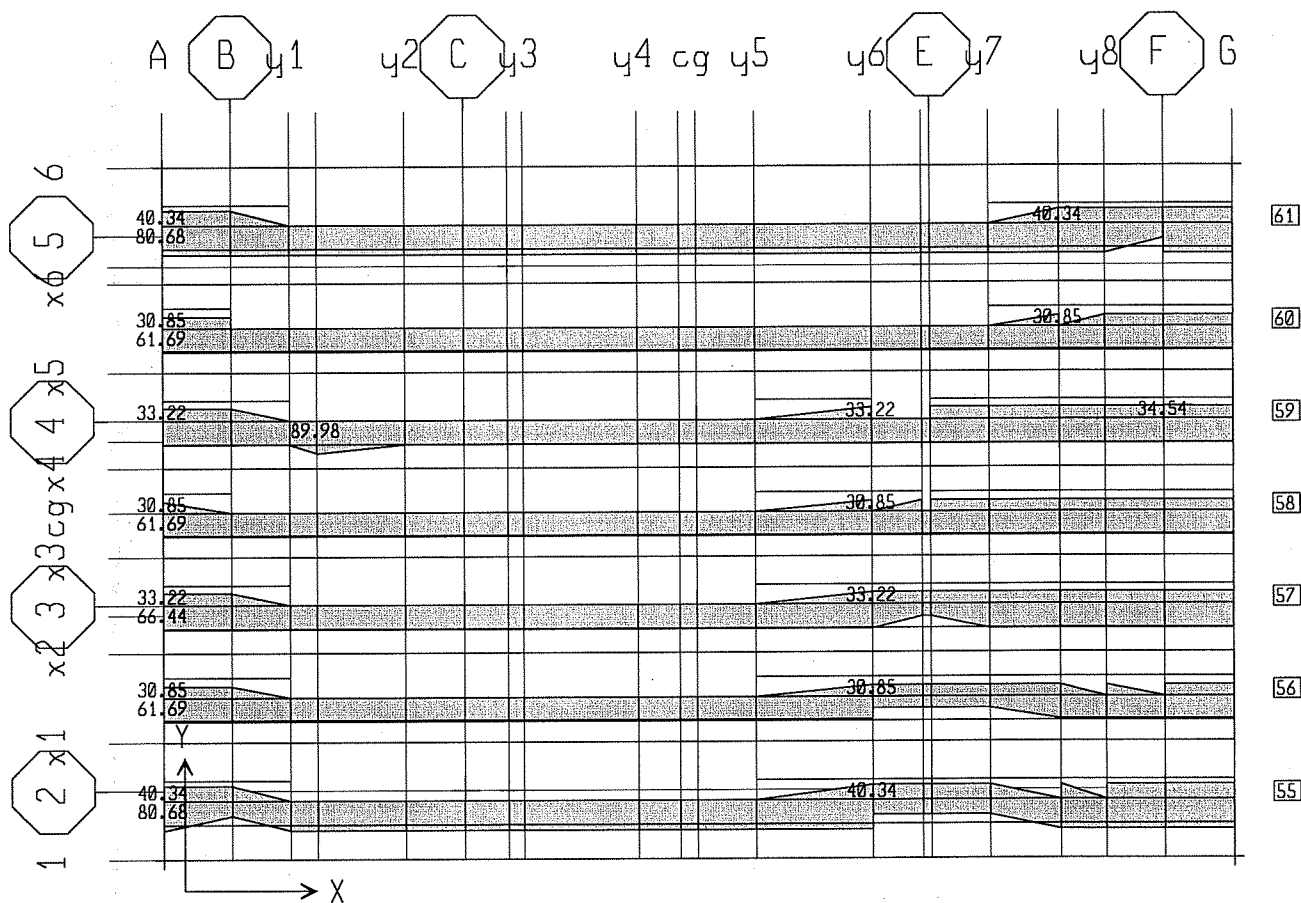
Point Label	DL [ton]	LL [ton]	DL+LL [ton]	Lx [m]	Lx*[DL]	Lx*[DL+LL]	Ly [m]	Ly*[DL+LL]
600	138	28	166	2.75	380	457	0.00	0
601	144	36	173	12.35	1,778	2,137	0.00	0
616	240	75	290	3.75	900	1,088	2.50	725
617	394	109	477	5.25	2,069	2,504	2.50	1,193
618	433	118	524	12.35	5,348	6,471	2.50	1,310
C179	896	266	1,159	0.00	0	0	5.00	5,795
630	224	84	275	2.75	616	756	5.00	1,375
631	242	87	299	5.25	1,271	1,570	5.00	1,495
632	313	110	388	8.75	2,739	3,395	5.00	1,940
633	149	51	184	9.95	1,483	1,831	5.00	920
634	80	32	99	12.35	988	1,223	5.00	495
640	121	47	148	2.75	333	407	7.50	1,110
642	111	10	136	8.75	971	1,190	7.50	1,020
Σ =	3,485	1,053	4,318		18,874	23,028		17,378
				Lx avg =	5.42	5.33	Ly avg =	4.02

Provide Bore Pile Dia. 1000 mm. = 20  
 SW : 15.90x9.60x2.5x2.4 = 916 ton  
 Load / Pile = 262 ton



## Joint Reaction Diagram

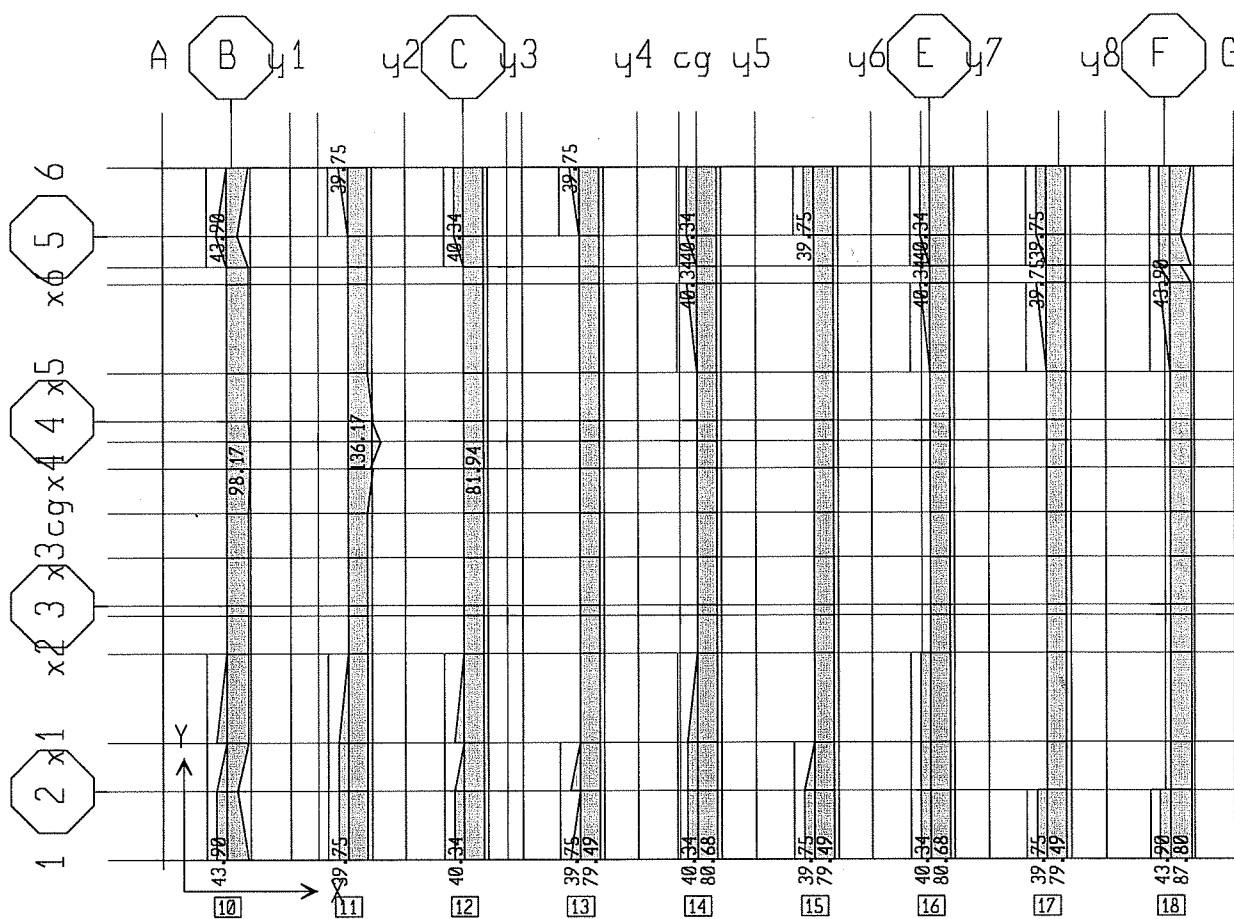
# X-Strip Reinforcement



Top. Reinf.:  $A_{sreq} = 40.4 \times 7 = 283 \text{ cm}^2$   
 $A_{smin} = 0.0015 \times 960 \times 250 = 360 \text{ cm}^2$  } 540032 (T)

Bot. Reinf.:  $A_{sreq} = 80.68 \times 7 = 565 \text{ cm}^2 = \underline{\underline{2 \times 42 + 0.8 \times 2 (B)}}$

# Y-Strip Reinforcement



Top Reinf:

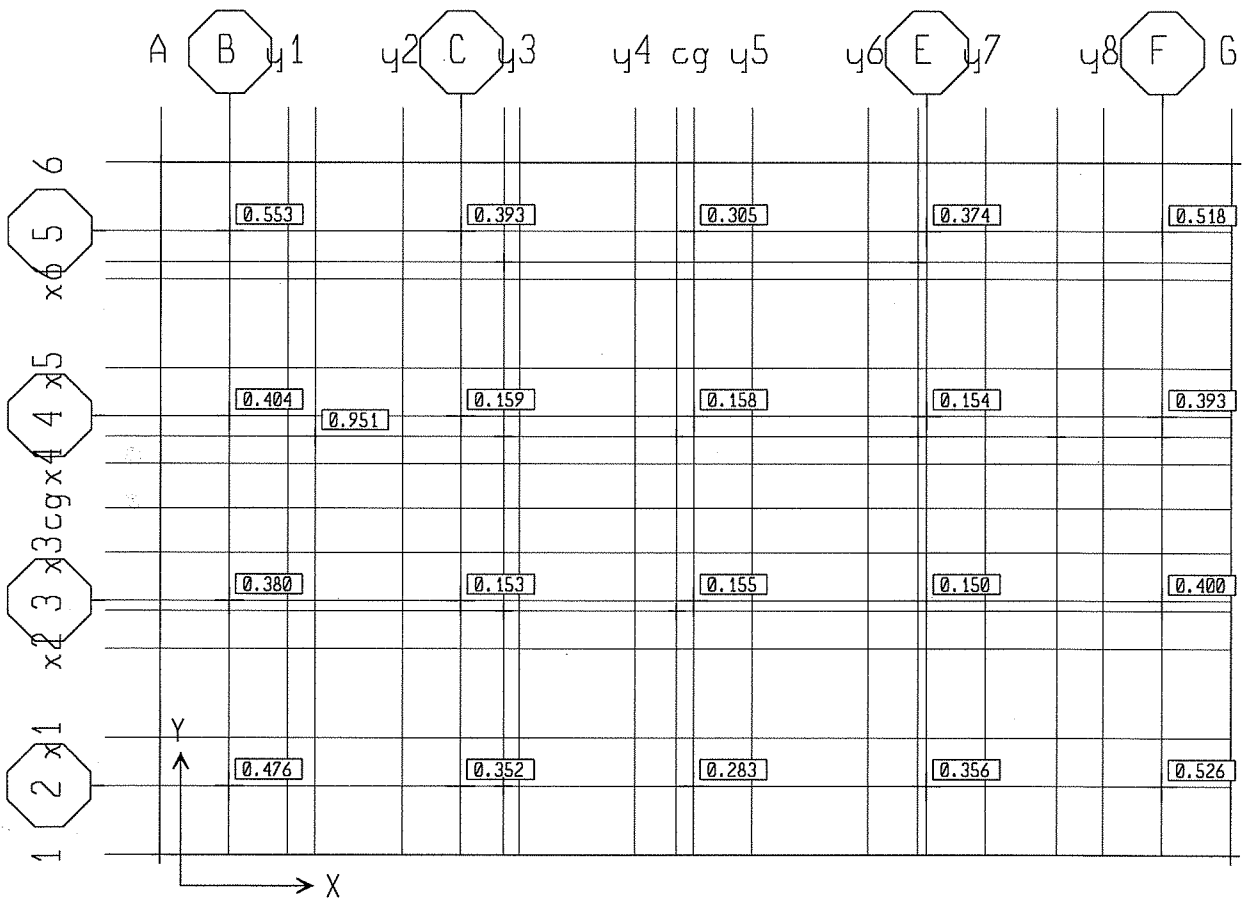
$$A_{streq} = 43.9 \times 9 = 396 \text{ cm}^2$$

$$A_{stim} = 0.0015 \times 1590 \times 250 = 592 \text{ cm}^2$$

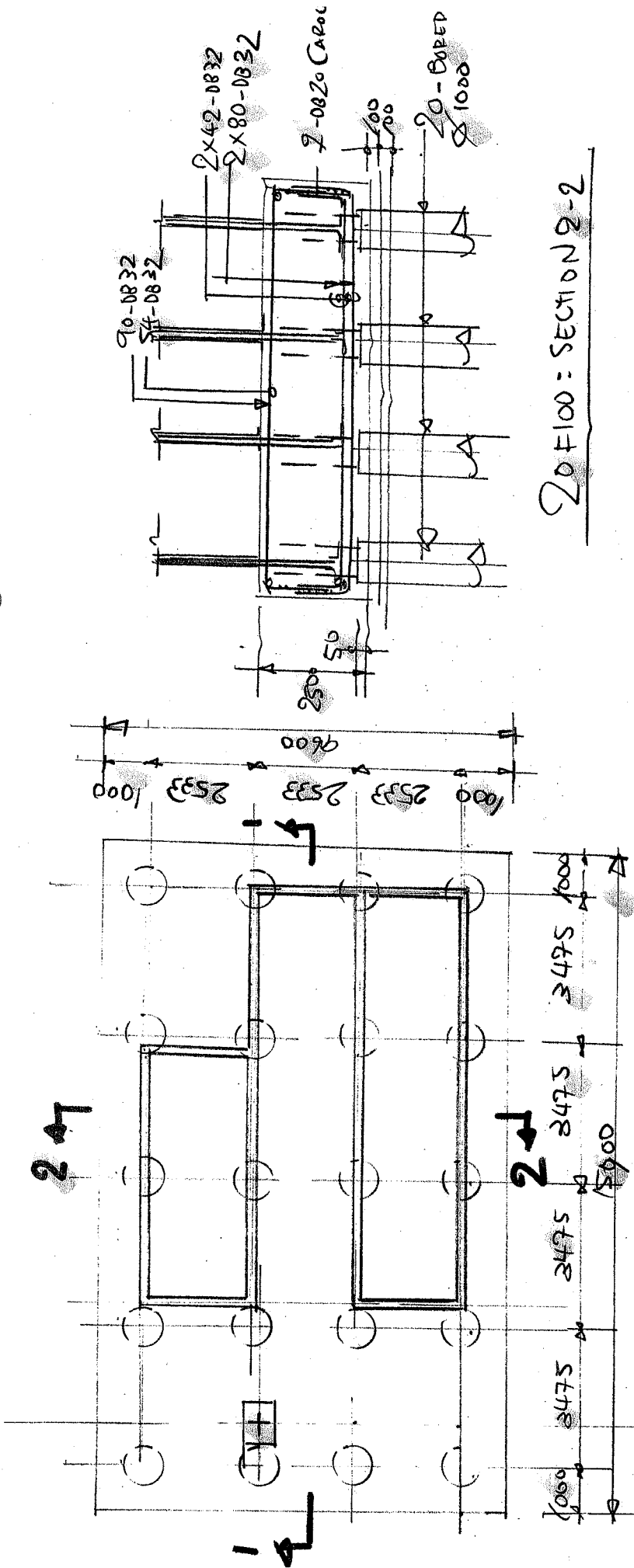
90-0032 (T)

Bot. Reinf:

$$A_{streq} = 136.12 \times 9 = 1225 \text{ cm}^2 = 2 \times 80-0032 (\text{B})$$



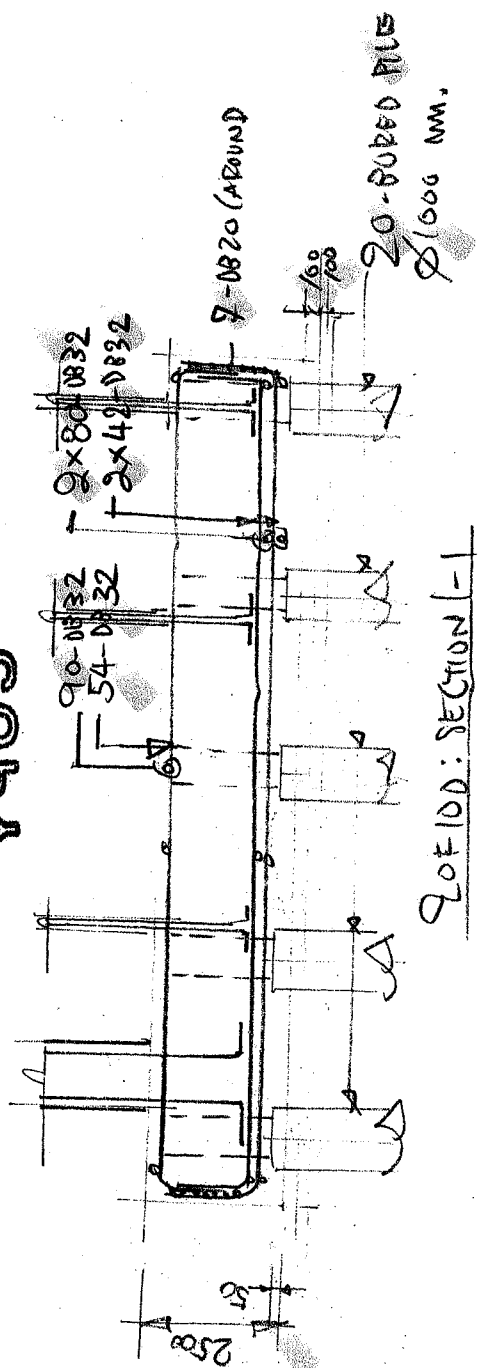
Punching Shear Capacity Ratios



20F100 = SECTION 2-2

20F100: PLAN.

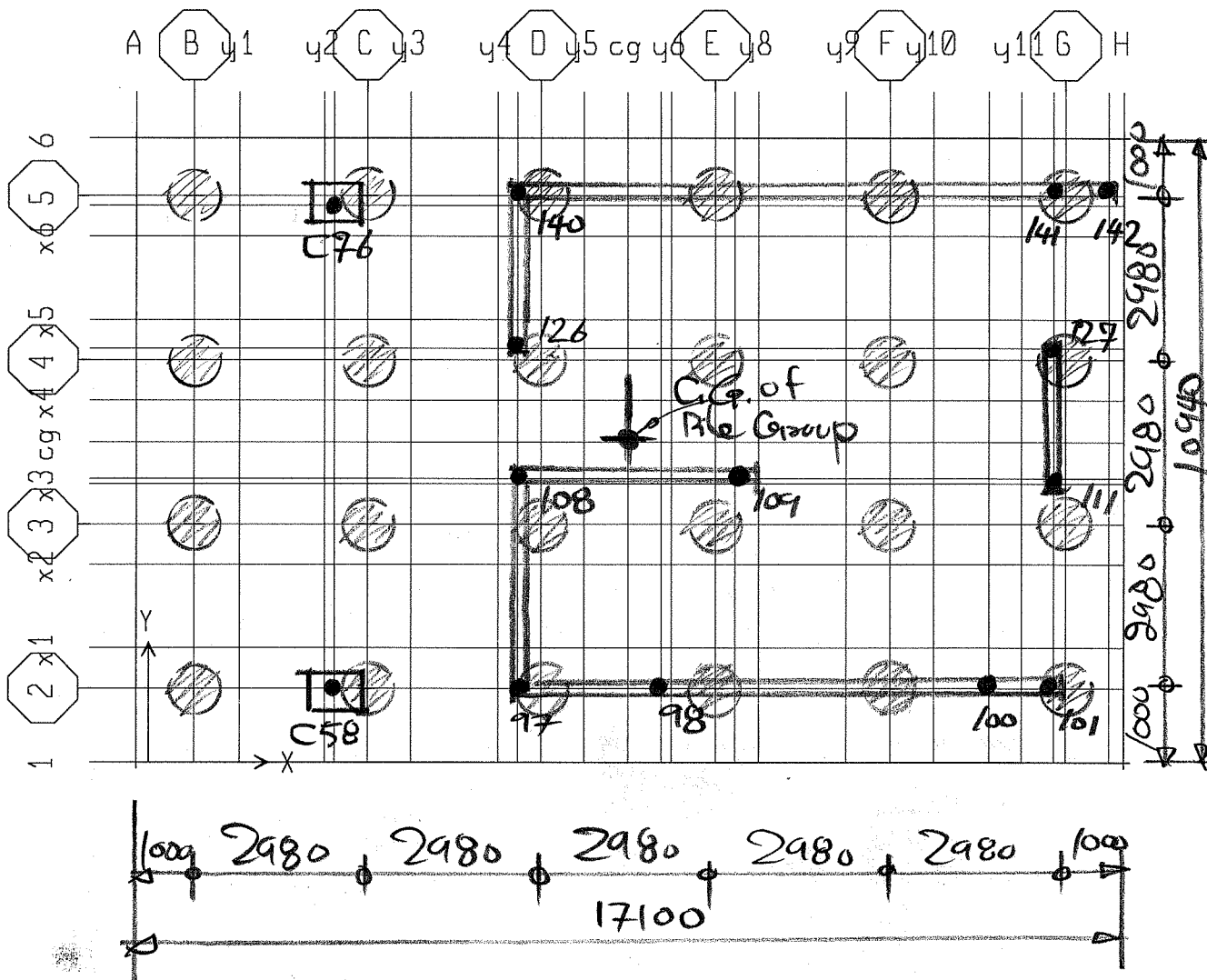
**Y9903**



20F100: SECTION 1-1

# Design of 24-F100

Provided Pile Cap : 17.10x10.94x2.50 dp



24-F100 : PLAN

# Loading from Analysis

Reaction : 24-F100

Point Label	DL [ton]	LL [ton]	DL+LL [ton]	Lx [m]	Lx*[DL+LL]	Ly [m]	Ly*[DL+LL]
C58	851	247	1,098	0.00	0	0.28	302
97	328	73	401	3.20	1,283	0.00	0
98	397	88	485	5.68	2,755	0.00	0
100	366	79	445	11.30	5,029	0.00	0
101	59	13	72	12.40	893	0.00	0
108	376	81	457	3.20	1,462	4.00	1,828
109	182	39	221	6.95	1,536	4.00	884
111	187	45	232	12.40	2,877	4.00	928
126	143	28	171	3.20	547	6.15	1,052
127	188	46	234	12.40	2,902	6.15	1,439
C76	775	221	996	0.00	0	9.20	9,163
140	543	111	654	3.20	2,093	9.48	6,197
141	421	84	505	11.30	5,707	9.48	4,785
142	41	11	52	13.20	686	9.48	493
Σ =	4,857	1,166	6,023		27,769		27,070
				<b>Lx avg =</b>	<b>4.61</b>	<b>Ly avg =</b>	<b>4.49</b>

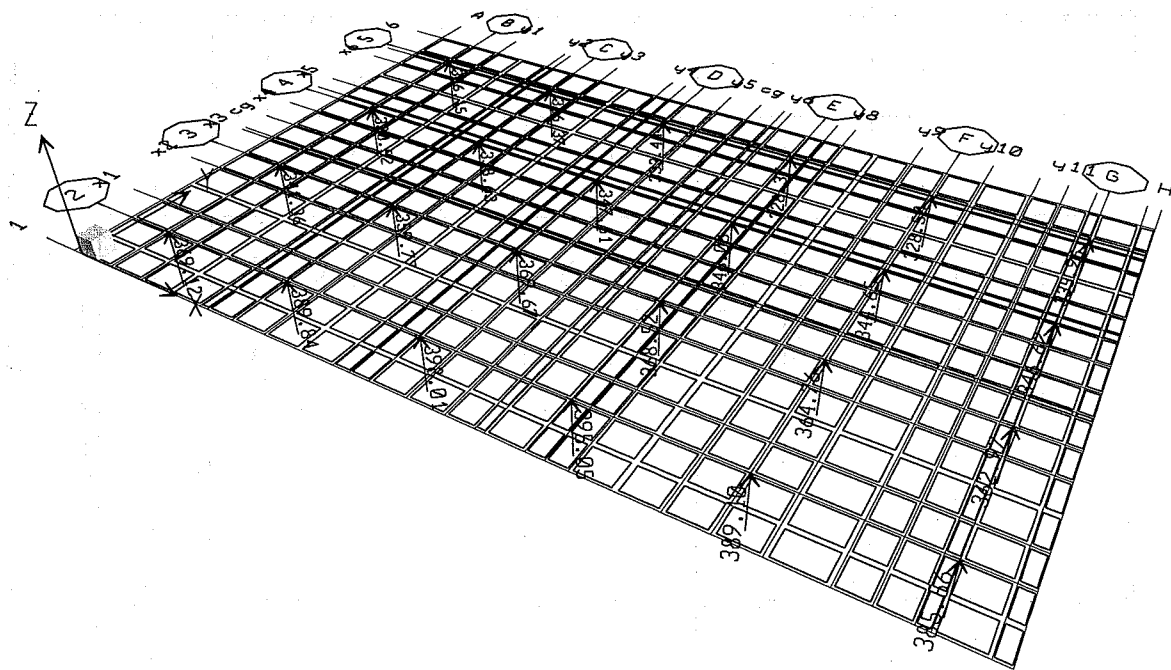
Provide Bore Pile Dia. 1000 mm.

SW : 17.10x10.94x2.5\*2.4

Load / Pile

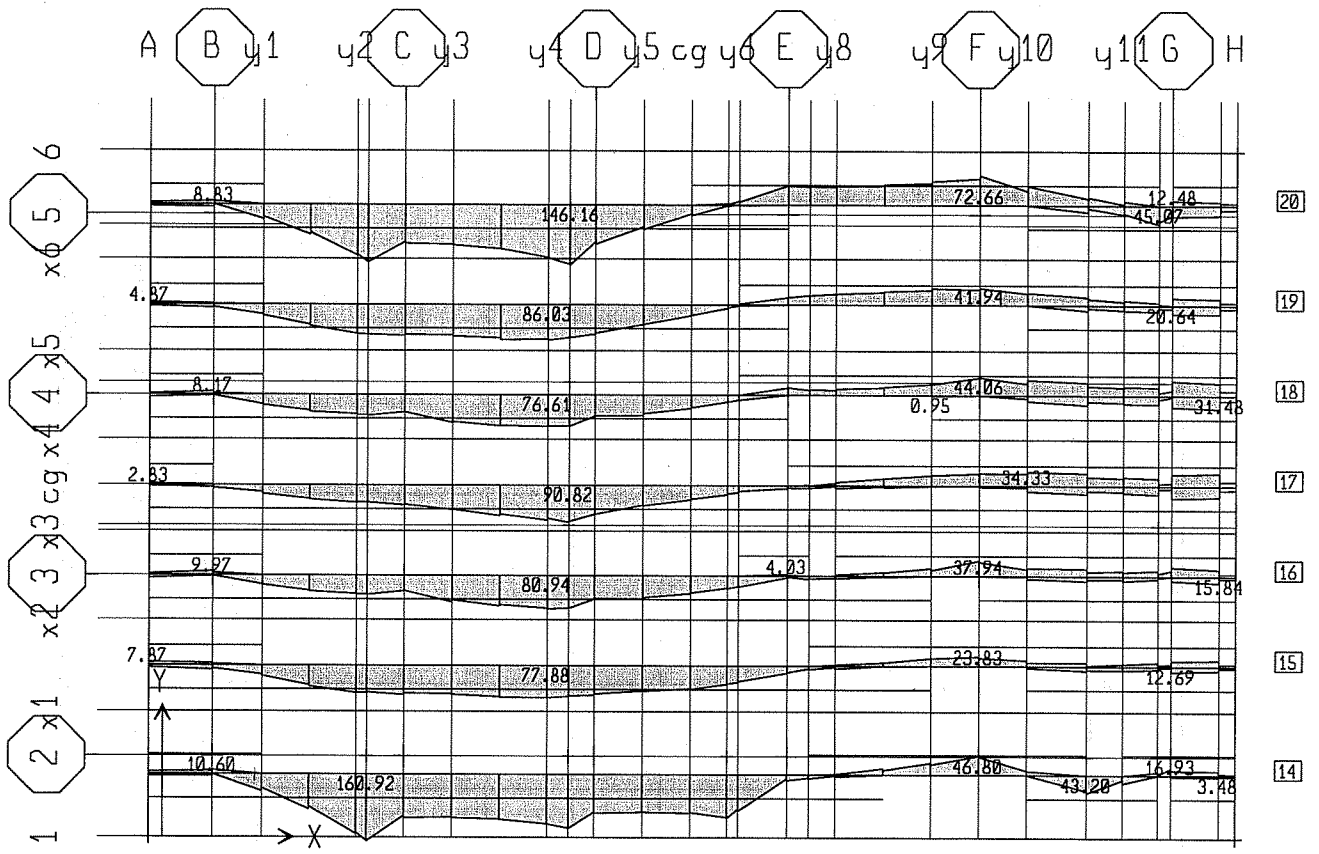
= 24  
 = 1,122 ton  
 = 298 ton





## Joint Reaction Diagram

# X-Strip Reinforcement



Top. Reinf:

$$A_{s, req} = 92.66 \times 7 = 508 \text{ cm}^2$$

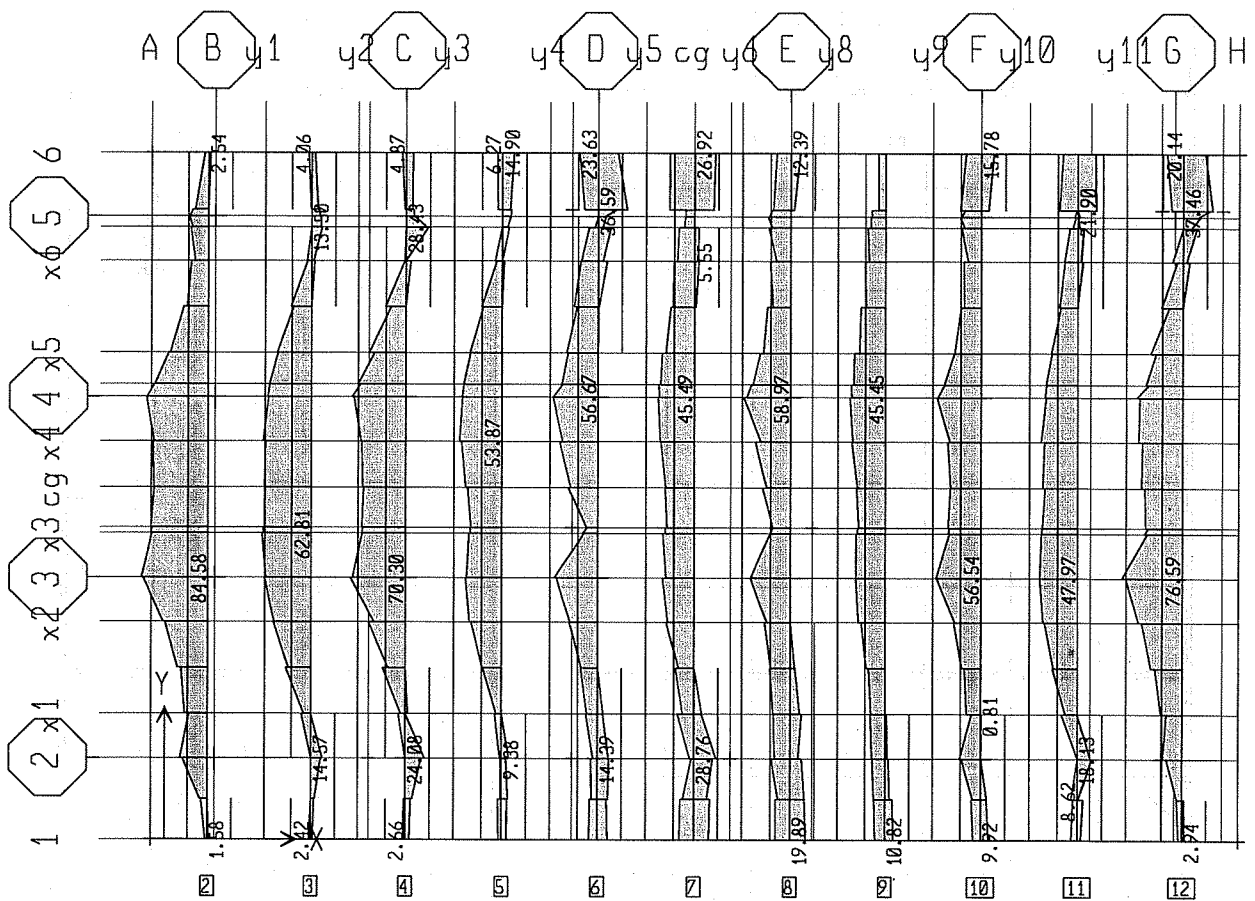
$$A_{s, min} = 0.0015 \times 1094 \times 250 = 410 \text{ cm}^2$$

} 65-DB32 (T)

Bot. Reinf:

$$A_{s, req} = 160.92 \times 7 = 1127 \text{ cm}^2 : \underline{2 \times 70DB32 (B)}$$

# Y-Strip Reinforcement

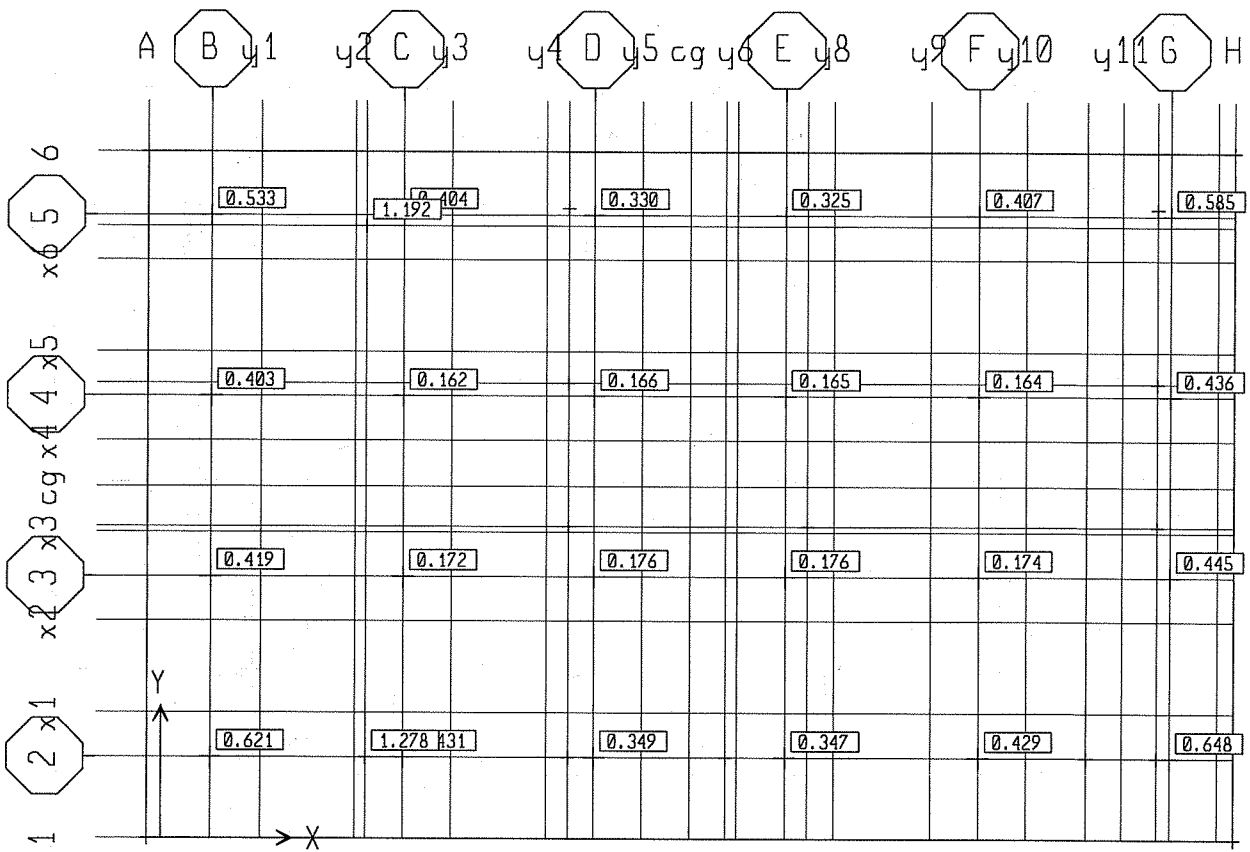


Top. Reinf:

$$\left. \begin{aligned}
 A_{st \text{ req}} &= 84.58 \times 11 = 931 \text{ cm}^2 \\
 A_{st \text{ min}} &= 0.0015 \times 1710 \times 250 = 642 \text{ cm}^2
 \end{aligned} \right\} = 120-0832 \text{ (7)}$$

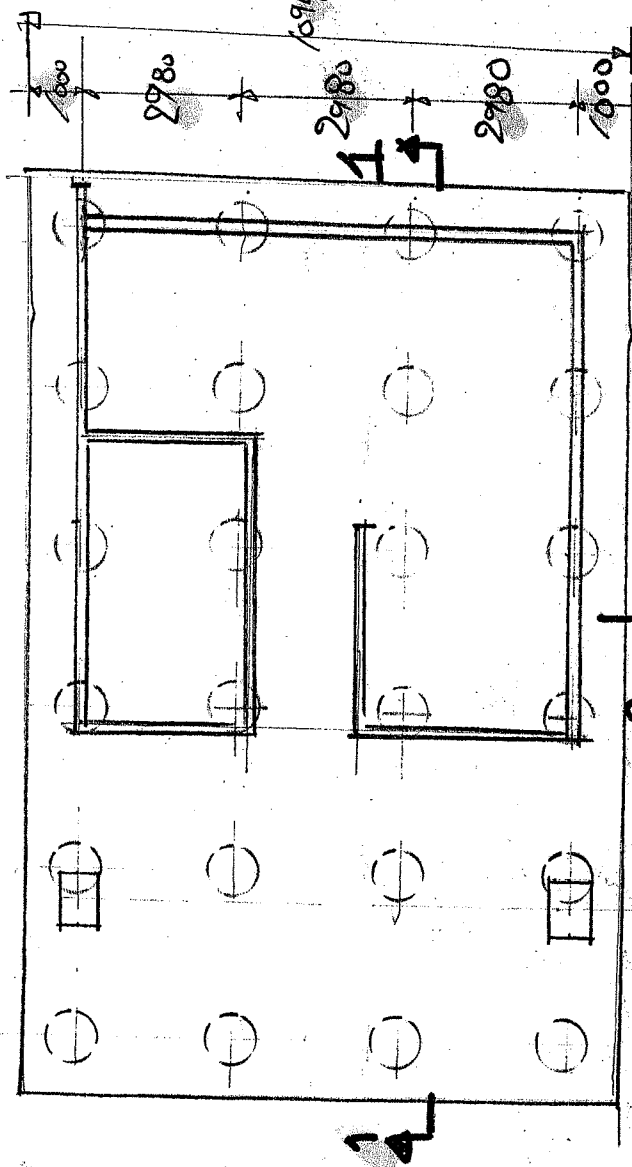
Bot. Reinf:

$$A_{sb \text{ req}} = 37.46 \times 11 = 413 \text{ cm}^2 = 120-0832 \text{ (8)}$$



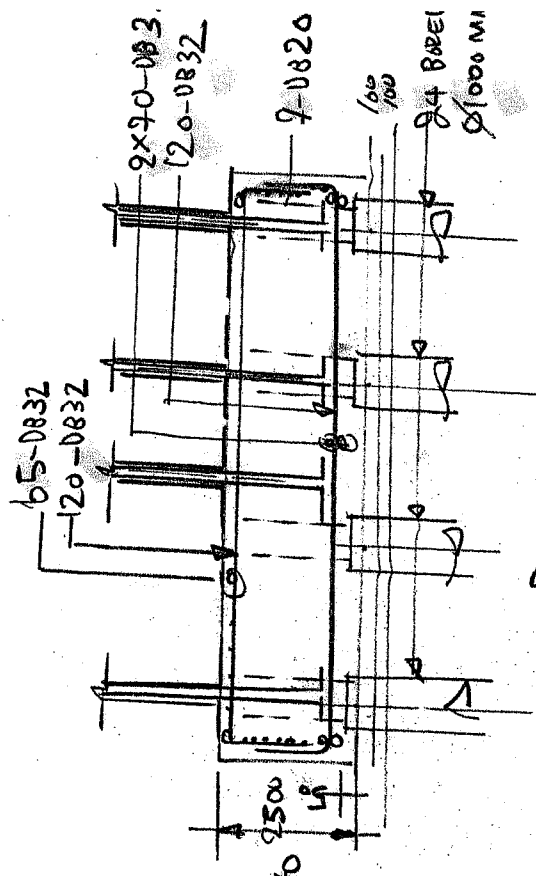
## Punching Shear Capacity Ratios

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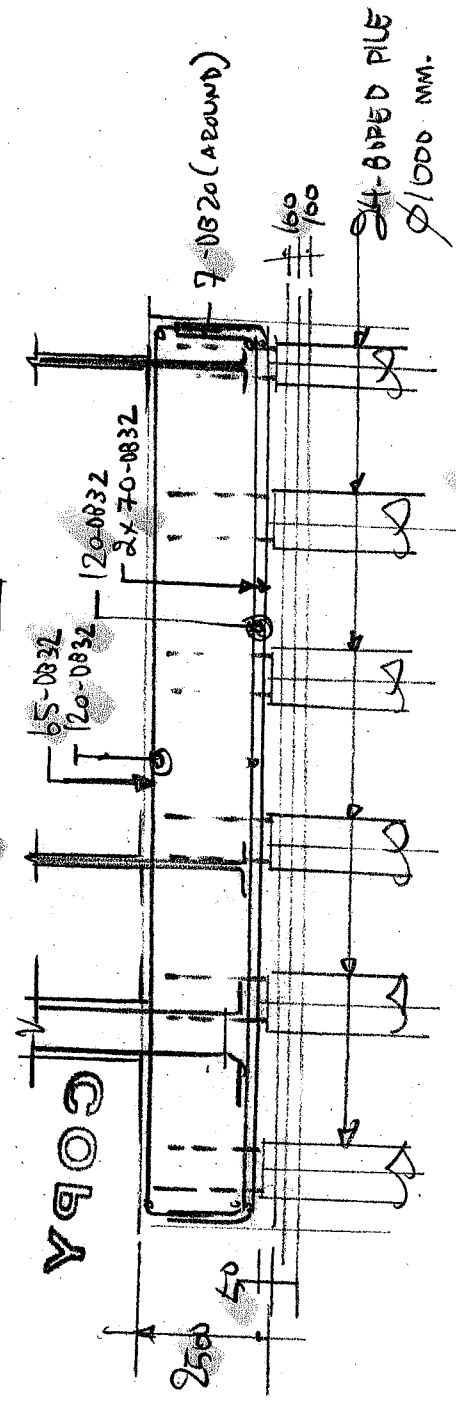


16000 | 2980 | 2980 | 2980 | 2980 | 16000  
 7100

24F100: PLAN.



24F100: SECTION 2-2



24F100: SECTION 1-1